## INTEGRATING DIFFERENTIAL GLOBAL POSITIONING SYSTEMS AND GEOGRAPHIC INFORMATION SYSTEMS FOR ANALYSIS AND MAPPING OF SKELETAL DISPERSALS

by

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## ABSTRACT

Scene mapping is an integral part of processing a forensic scene with scattered human remains. By utilizing the appropriate mapping technique, investigators can accurately document the location of human remains and maintain a precise geospatial record of this evidence at a scene. Global positioning system (GPS) units have been used for years to survey the spatial distribution of large-scale archaeological sites. However, differential global positioning (DGPS) unit now provide decreased positional error suitable for small-scale surveys, such as forensic scenes. Because of the lack of knowledge concerning this utility in mapping a scene, controlled research is necessary to determine the practicality of using DGPS in mapping scattered human remains in different environments. The purpose of this research is to quantify the accuracy of a DGPS unit for mapping skeletal dispersals and to determine the applicability of this utility in mapping dispersed remains. First, the accuracy of the DGPS unit was determined using known survey markers in different environments. Secondly, several simulated scenes were constructed and mapped in open, tree-covered, and structure-obstructed environments using the DGPS. Factors considered included the extent of the dispersal, data collection time, and the use of offsets. Data were differentially postprocessed and compared in a geographic information system (GIS) to evaluate the most efficient recordation methods. Results of this study show that the DGPS is a viable option for mapping human remains in open areas. Furthermore, guidelines for accurate scene mapping using a DGPS unit will be provided, along with a discussion concerning the integration of DGPS into GIS for scene analysis and presentation.

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Dedicated to my fiancé Michael Stewart and my sister Raquel Walter, for keeping me grounded both here and there.

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## **CHAPTER ONE: INTRODUCTION**

The recent re-emphasis of methodological techniques in forensic archaeology is a development that has provided forensic anthropology with "a new conceptual framework, which is broader, deep, and more solidly entrenched in the natural sciences" (Dirkmaat et al., 2008:33). This shift is currently changing the goals of forensic anthropologists and how they approach situations in the field. Dirkmaat et al. (2008) attribute the current configuration of forensic anthropology to four developments: (1) improvement of field archaeology methods, (2) new technology, (3) new techniques used in the analysis of spatial data in the field, and (4) the emergence of recovery methods more geared toward forensic contexts. The implementation of technology in the field has resulted in the use of archaeological methods in a forensic setting. Site mapping has advanced from hand-drawn maps that note the distribution of evidence to the use of technology for recording specific locations of evidence and spatial data at the scene.

While standard global positioning systems (GPS) units generally do not offer the appropriate degree of accuracy for mapping (Listi et al., 2007), portable differential global positioning system (DGPS) units offer decimeter error margins which may be appropriate for mapping scattered remains. These enhanced units have the potential to collect accurate positional information of objects and provide the exact location of the object on the Earth. In instances where skeletal dispersal are widely scattered, a DGPS unit may be a useful tool. Thus, it is essential to determine the accuracy and practicality of using a mid-price DGPS unit in mapping skeletal dispersals and explore the value of DGPS in field recovery situations.

## Controlled Research

As forensic archaeology is becoming more integrated into forensic anthropology, controlled research is essential in determining the utility of innovative technology in a forensic context. Controlled research is a necessary method of testing controlled variables in order to ascertain the best use of equipment in the field (France et al., 1992). Data collection of known points, such as survey markers, in controlled environments can assess the accuracy of DGPS units. Moreover, controlled environments and simulated dispersals can be utilized to apply and evaluate the best methods for collecting point data in these environments.

Limited research has been conducted concerning the use of DGPS in the mapping of skeletal remains. Listi et al. (2007) determined that the use of a mid-price traditional GPS receiver is not as accurate as traditional mapping techniques because of limiting factors such as tree cover density, proximity of remains to structures and trees, and the positioning of satellites which can result in erratic data. This preliminary research underlines the necessity of assembled scenarios wherein different settings and variables can be controlled and tested to assess the accuracy of using a DGPS in mapping skeletal dispersals. Additionally, the determination of accuracy of a DGPS in different environmental scenarios is necessary to evaluate the practicality of using this utility in mapping human remains. Furthermore, research has not been conducted concerning the use of georeferenced data in skeletal dispersals. The integration of exact coordinates to survey data is ideal, as this provides a highly accurate record of the scene, along with the ability to manipulate survey data in a geographic information system (GIS) to maintain geospatial information (Wu et al., 2004).

Finally, the use of GIS in forensic investigations has generally been limited to crime mapping, rather than with individual scenes (Spencer et al., 2003; Manhein et al., 2006). It is because of this lack of published research that it is necessary to utilize these new technologies to determine the advantages and disadvantages in the field. Additionally, the practicality of using DGPS units in different environments can aid in the development of efficient and accurate methods for data collection, maximizing point accuracy and information of skeletal elements and associated evidence at a scene. Thus, controlled research concerning different mapping technologies in different environmental scenarios is essential in order to expand knowledge of new technology in the mapping of scattered remains. Furthermore, it is necessary to experiment with these technologies to determine their practicality and applicability in the field, supporting the current shift of integrating new technology and techniques in forensic archaeology.

#### **Research Objectives**

The primary objective of this research is to compare different data collection techniques using the DGPS in the mapping of simulated skeletal dispersals in varying scenarios and to discuss the benefits of mapping these scenes using the DGPS with the integration of GIS for data analysis and presentation. This research will (1) determine the accuracy of using a DGPS unit in differential environmental scenarios; (2) construct different scenarios in order to simulate scenes that may be encountered in real-life forensic cases; (3) collect geospatial and attribute data of features from skeletal dispersals using the DGPS; (4) process, analyze, and generate maps of the data in GIS; and (5) discuss the benefits, disadvantages, and methods of using DGPS and GIS for scene mapping of skeletal dispersals in different scenarios. The final chapter of this thesis will summarize the findings of the research conducted. The results of this study will contribute to the formulation of guidelines for using a DGPS unit in mapping skeletal dispersals and integrating the DGPS data into a GIS.

## Thesis Outline

This thesis will be divided into four chapters: the first chapter will provide an introduction into the research project; the second chapter will determine the accuracy of the DGPS unit in different environments; the third chapter will determine the practicality and accuracy of using a DGPS by constructing simulated scenarios, and the fourth chapter will discuss the integration of DGPS data into a GIS for analysis and mapping. The final chapter will also provide guidelines for using a DGPS unit when mapping scattered human remains.

## CHAPTER TWO: ACCURACY DETERMINATION OF DGPS UNIT USING SURVEY MARKERS

## **Introduction**

Accuracy is how close a value is to the true value, while precision refers to the way in which the data is measured or stored (Wheatley and Gillings, 2002). Unfortunately, accuracy is often difficult to measure as the true value is generally not known, except by the data collected. With precision, the more advanced or finer the unit of measurement that can be measured by an instrument, the more precise the data is said to be. Both accuracy and precision are important for recording a scene with forensic significance; however, for reconstruction purposes, accuracy is generally more important in forensic investigations (Gardner, 2004).

Innovative technology of DGPS units has enabled investigators to attain a higher level of accuracy than ever before. A high level of accuracy is essential particularly for scenes involving human remains. The accuracy of DGPS units has increased over the years from advancements of spatial technology; however, even the most advanced DGPS units cannot control certain factors during acquisition of positional information. Thus, it is imperative to recognize these limitations and integrate this into data collection and analysis.

Though extensive research has been conducted to determine the influence of certain variables on the accuracy of GPS, limited research has been conducted concerning the use of DGPS in the mapping of human remains to ascertain the effect of these variables. Listi et al. (2007) assessed the use of a standard GPS unit with the addition of a beacon receiver for mapping scattered human remains at a scene. The authors determined that the low-priced GPS unit generated a positional error of less than one-half meter; however, the GPS unit could not

distinguish features that were in close proximity to each other when data was collected with a 100-second collection time. As with their previous study (Listi et al., 2003), the authors concluded that using a GPS was not as reliable as traditional mapping techniques. It must be noted, however, that the DGPS receiver used by the authors has become obsolete and models that are more accurate have been developed in the last four years that offer decimeter accuracy with postprocessing. Thus, it is crucial that the accuracy of these new and enhanced DGPS units be assessed within multiple environmental scenarios so that these error determinations may be applied in the field.

#### Purpose

The purpose of this chapter is to determine the accuracy of the DGPS unit in different environments by collecting point data at known survey markers for 50-second and 100-second collection times in various settings. Additionally, the distance accuracy of the DGPS unit will be determined by comparing maximum distance measurements of long bones with collected point data of the long bones at proximal and distal ends. Furthermore, the determination of collecting proximate bones as separate features or as a single feature was also considered. Bones were measured at distances of 5 cm, 10 cm, 15 cm, 20 cm, 25 cm, and 30 cm to determine the best data collection method of clustered skeletal elements.

## Differential Global Positioning Systems Theory

To understand the variables that affect the accuracy of a GPS unit during data collection, it is first important to understand the components and basic mechanics of GPS. Global positioning systems is a satellite-based positioning system involving 24 satellites circling the earth (El-Rabbany, 2006). A GPS receiver uses positional information from these satellites to calculate the position of an object on the earth (El-Rabbany, 2006). The development of a differential global positioning system (DGPS) allows more accurate point positions, which may be utilized to document the position of specific objects, such as skeletal elements and additional features, at a scene.

A DGPS unit is a more accurate enhancement of a standard GPS unit that requires two receivers; one remains stationary while the other records positional data (Figure 1). The stationary receiver, a basestation, relates all of the satellite measurements onto a single local reference (El-Rabbany, 2006; Napton and Greathouse, 2009). The basestation measures the timing errors and provides correction information to the other receiver during postprocessing. Differential postprocessing software obtains known basestation information via the internet and then compares this information to the mapped point data (Figure 2) for increased positional accuracy (Spencer et al., 2003). Furthermore, DGPS units are handheld units that are compact and easy to transport to and from the scene, and only a single operator is necessary to collect positional information (Napton and Greathouse, 2009).



Figure 1- Differential GPS positioning



Figure 2- Illustration of postprocessing for differential GPS correction

Sando et al. (2005) categorize GPS receivers into three main configurations in the United States (Table 1): autonomous, wide area augmented system (WAAS), and continuously operating reference station (CORS). Autonomous configuration is the simplest mode, using a single receiver and three satellites to determine the current location. Wide area augmented system uses both land-based control stations and geostationary satellites to adjust GPS data to improve accuracy and was developed in order to provide accurate aircraft navigation. This technology is a correction service that was created at no extra charge for all WAAS enabled GPS receivers to achieve 3.0 m accuracy or less with multiple fixes and 7.0 m accuracy with single fixes (Bolstad, 2005; Sando et al., 2005). This system was originally developed for military use and has recently become available to the general public when using a GPS enabled with NAVSTAR technology (Bolstad, 2005). Like differential postprocessing, signals from satellites are received by ground reference stations through North America and correction information is calculated and then broadcasted to GPS units with NAVSTAR technology automatically. Currently, two WAAS satellites are in place over the Atlantic and Pacific Oceans (Sando et al., 2005).

The configuration used for this project, the CORS network, provides the most accurate positional information. The CORS network is available to the public in order to improve the precision of collected positions (Bolstad, 2005). Positional data is collected from several basestations in the CORS network and is used to differential correct positional data collected in the field or with postprocessing. Postprocessed CORS data requires differential processing against CORS basestation data that is retrieved from the internet. Data collected via CORS configuration can also be used for real-time correction if the GPS unit has the capability of connecting to the internet in the field. Any DGPS receiver within the range of the radio beacons can access the CORS network with either an antenna or additional beacon receiver. According to Bolstad (2005), the federal government is looking to extend the CORS network to all areas of

the United States, but as of now, only certain locations can use CORS technology because of the

limited locations of CORS radio beacons.

|   | Autonomous  | Wide Area Augmented System<br>(WAAS)   | Continuously Operating<br>Reference Station (CORS)  |
|---|---|--|---|
| • | Simplest mode<br>Least accurate<br>Uses single receiver<br>and 3 satellites | <ul> <li>Free correction service from<br/>the US military</li> <li>Uses land-based control<br/>stations and geostationary<br/>satellites</li> <li>Used to achieve at least 3.0 m<br/>accuracy with multiple fixes</li> <li>Available to GPS units with<br/>NAVSTAR technology</li> </ul> | <ul> <li>Most accurate</li> <li>Requires postprocessing</li> <li>CORS data available over<br/>the internet</li> <li>Can be used for real-time<br/>correction</li> <li>Not available in all areas<br/>of the US</li> </ul> |

Table 1- Summary of the three main GPS configurations used in the United States

Several influences can limit the accuracy of points recorded by a DGPS receiver such as cloud cover, satellite position, and obstruction of satellites from buildings and tree cover (Sando et al., 2005). Prior to 2000, selective availability was a heavily influential factor in the inaccuracy of DGPS positional data. Selective availability was a protective feature imposed by the United States Department of Defense that artificially deteriorated clock and ephemeris data for civil users (Bolstad, 2005). In 2000, President Clinton requested that selective availability was lifted by the federal government in May of 2000, studies have been conducted to ascertain the improvement of accuracy by GPS units (Graettinger et al., 2001). Graettinger et al. (2000) reported an improvement in accuracy up to 10-fold after the removal of selective availability. Furthermore, a three year study by Sando et al. (2005) demonstrated considerably higher accuracy when compared with the Graettinger et al. (2001) study.

Though Selective Availability no long produces inaccuracies during the acquisition of positional information, several other sources of error still occur. Ionospheric and atmosphereic delays can introduce error when satellite signals travel through the ionosphere and atmosphere. Changes in charged particle density in the ionsphere and changes of atmospheric density from temperature change in the atmosphere can affect the travel speed of the satellite signals (Lechner and Baumann, 2000). Differential GPS units, however, use dual frequency receivers to differentially correct this information by comparing the information collected by the receiver and the basestation take into account these changes and create sophisticated base models to reduce error (Bolstad, 2005). An almost negligible source of error can be from unsychronization of the atomic clocks on the satellite. This, however, is also corrected in postprocessing (Bolstad, 2005; Trimble Navigation Limited, 2009).

The geometry of satellites positions can also affect the positional error of a DGPS receiver. Satellites are most accurate when they are spaced farther apart, as close-set satellites overlap, causing areas of positional uncertainty when signals intersect (Bolstad, 2005). The geometry of a constellation of satellites is expressed by a number called the Dilution of Precision (DOP). Types of DOP include, Vertical DOP (VDOP), Horizontal DOP (HDOP), and Positional DOP (PDOP). Positional DOP is the most commonly used in the determination of complementary satellite geometry and is defined as the "ratio of the volume of a tetrahedron created by the four most widespread, observed satellite to the volume defined by the ideal tetrahedron" (Bolstad, 2005:183). The composition of an ideal tetrahedron includes one overhead satellite and three surrounding satellites spaced at approximately 120-degree intervals. The PDOP is expressed as a number with the ideal tetrahedron being 1. The closer the satellites

are to each other, the less accurate the satellite geometry, increasing the PDOP number (Johnson and Barton, 2004). Thus, a lower PDOP is more desirable. DGPS receivers will automatically choose the satellite constellation with the lowest PDOP. Positional DOP is predetermined and can be acquired using planning software before data is collected.

Multipath signals are the most common source of error in standard GPS and DGPS units. Multipath signals are signals from satellites that are reflected off of obstructions between the receiver and the satellite such as clouds, trees, and structures (Lechner and Baumann, 2000). Because these signals are reflected, the signals travel a further distance than direct satellite signals, introducing an offset into satellite positions (Lechner and Baumann, 2000). These multipath signals are also usually screened out by antennae, but can still influence point collection (Bolstad, 2005).

#### Materials and Methods

The differential GPS unit used for this research was a Trimble GeoExplorer 2008 Series GeoXH handheld differential GPS receiver with Zephyr antenna (Figure 3). The receiver uses a field computer powered by Microsoft Windows Version 6 operating system and Terrasync software. The receiver uses both H-star and EVEREST multipath technology to provide heightened accuracy after postprocessing using the internal antenna. The addition of the external Zephyr antenna provides better locational recordation with 10 cm to 30 cm accuracy when data is differentially postprocessed (Trimble Navigation Limited, 2009). Interestingly, in 2005, Sando et al. found that when an older model of this DGPS unit was compared to three similar receivers, the Trimble GeoExplorer receiver was the most accurate.



Figure 3- Figure of DGPS and antenna with labeled components

The use of an external antenna increases the accuracy of GPS data in three ways (J. Robeson, personal communication, March 12, 2012). First, by placing the antenna on a rangepole, the antenna is anchored to an associated point on the ground that holds the configuration in plumb. The operator is, thus, given a definitive point on the ground that they intend to map. Secondly, by placing the antenna above the operator, the GPS and its accuracy are not suffering from the abstraction the operator creates with his or her own body. Finally, by

using an antenna that is of a higher quality, such as the Zephyr, the quality and strength of the signal is increased drastically. The Zephyr also adds an additional frequency antenna to the single frequency DGPS unit. The reception of both frequencies creates a higher accuracy position due to the GPS receiver and software's ability to process both frequencies per satellite. These frequencies are typically associated with survey-grade DGPS units because of its ability to calculate a far higher level of precision.

## **Controlled Points**

Survey markers, or benchmarks, are known points on the earth maintained by various federal and state agencies such as the Department of Transportation (Bolstad, 2005; Sando et al., 2005; Dupras et al., 2011). It is common to find these points at road intersections, city centers, and other areas of interest, as these points are the basis for defining property boundaries. The exact coordinates, location, and description of these survey markers may be obtained from the government for various reasons. Currently, survey markers are most often determined using high-precision GNSS technologies, such as commercial-grade DGPS units and are accurate to the sub-centimeter (Bolstad, 2005; Sando et al., 2005).

Survey markers have previously been used in civil engineering studies to ascertain the error of various GPS units (Graettinger et al., 2001 and Sando et al., 2005). This study will also utilize survey markers as known points on the earth to determine the error of the Trimble GeoExplorer 2008 Series DGPS unit. The coordinates of the known points used for this project were provided by the Department of Transportation of the State of Florida and consist of 2 points

in open areas, 1 point in an area under tree cover, and 1 point in an area near a tall structure (Figure 4). The survey markers were located within 5 kilometers of each other on New York Ave. in Deland, Florida (Figure 5). See Appendix A for survey marker information provided by the Department of Transportation.



Figure 4- Images of reference markers for survey markers 90501004, 90501007, VOLV119, and 90501006 provided by the Florida Department of Transportation



Figure 5- Labeled map of survey markers with environment type in Deland, FL

#### Distance Accuracy

In addition to the aforementioned survey marker scenarios, the mapping of proximate bones as separate features or as clusters were analyzed by mapping bones at 5 cm, 10 cm, 15 cm, 20 cm, 25 cm, and 30 cm distances. The 5 cm distances were chosen to consider the different levels of clustering and to apply these systematic distances to long bone lengths. Additionally, the accuracy of the DGPS unit in mapping long bones will be determined by comparing maximum distance measurements of long bones with collected point data of the bones at proximal and distal ends at both 50-second and 100-second collection times. The results from the analysis of these data will then be applied to data collection for the simulated scenarios in Chapter 3.

## Data Collection

Prior to the day of data collection, planning almanac software (available through Trimble) was consulted to determine the best time for data collection. This software provides information including the satellite position data, DOP data, and elevation data on specific days (Figure 6). The best time for data collection on a day was determined by considering the greatest number of satellites, with at least 4 satellites being the most desirable, and the least PDOP value, with values less than 2 being the most desirable (Johnson and Barton, 2004; Bolstad, 2005). Data collection was then conducted during this time period if weather permitted. See Appendix B for planning information for each day of data collection.

| Station Editor  |           |
|---|-----------|
| Station Name:   | ок        |
| Orlando,FL  | Cancel    |
| Position  | Apply     |
| Latitude:         N         1/28         1/28         1/28         1/27         1/27           Longitude:         W         1/28         1/27 | Delete    |
| Height: 0 [m]   | Obstacles |
| Elevation Cutoff: 10 *  | Map       |
| Time  | City      |
| Start Date:         1/29/2012            Start Time:         00.00            Duration:         24            Interval:         10  | Today     |
| Time Zone:<br>Eastern Standard Time<br>DST Difference UTC: -4.0 [h]   | Time Zone |

Figure 6 - Screenshot of data collection planning using Trimble Planning Software

In addition to planning the best time of day of data collection, the orientation of the DGPS receiver was also considered. Vertical orientation of the GPS receiver has been found to significantly influence accuracy, with vertical orientation of the receiver yielding coordinates that are more accurate, rather than horizontal orientation (Sando et al., 2005). Data were collected in US State Plane 1983, Florida East, with the NAD 1983 Conus datum, as this is the coordinate system and datum used by the Florida Department of Transportation in Deland, Florida. By using the same coordinate system and datum, additional error will not be introduced during processing and export from changes in the projection of the systems. Point data were collected using the batch method, which is the average of the point data in 1-second intervals. For example, a batch reading for a point collected at 1-second intervals for 50 seconds would yield the average of 50 points collected at that location. The collection times for this research

were chosen in accordance with the 100-second collection used in previous research (Listi et al., 2007), with the addition of the more efficient 50-second collection time for comparison purposes. The following information was recorded by the DGPS during point data collection:

- Date
- Time
- Northing
- Easting
- Max HDOP
- Max PDOP
- Correction Type
- Receiver Type
- Filtered and unfiltered positions
- Feature Name (*i.e.* Bone)
- Data dictionary used
- Filename

Point data were collected at each survey marker in 1-second intervals for 50 seconds and

100 seconds (Figure 7). Using this method, 25 points were collected consecutively for each

survey marker at both 50-second and 100-second time intervals. Fifty-second and 100-second

data were collected on different days because of time limitations that conflicted with planning

times. Further, data were collected using a predefined data dictionary with survey marker,

collection interval, environment type, and notes (Figure 8). These attributes were later exported

into ArcGIS with the point data for analysis and creating maps.



Figure 7- Point data collection using the DGPS unit at survey marker VOLV119

| NerraSync          | ©≓∢ ×                  | TerraSync            | Ş         | at d€ ok   |
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| 🗎 Data 🛛 🕶         | Û                      | 🗎 Data 🛛 🔫           | Û         |            |
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|                    |                        | 1 Bone               | 📕 ОК      | Cancel     |
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|                    |                        | Aspect:              | [         | Proximal 🔻 |
|                    |                        | Time:                | 11:39:15  | am 🔻       |
| ×<br>Point_generic | Line_generic           | Date:                | 2/1/2     | .2 🔻       |
|                    |                        | Collection interval: |           |            |
| 0                  |                        | File name:           |           | 50 s       |
| Area_generic       |                        |                      |           | 100 s      |
|                    |                        |                      |           |            |
|                    |                        | ок                   |           | Log        |

Figure 8- Screenshot of data dictionary used during data collection in Terrasync

## **Data Processing**

After point data were collected in the field, unprocessed data were transferred to a desktop computer using ActiveSync. This data was then imported into Pathfinder Office for differential postprocessing (Figure 9). Postprocessing capabilities refer to the ability of the GPS receiver to store GPS system data in a format that can be used to compute differential corrections of the location data using corrections recorded at a reference receiver to improve locational accuracy (Bolstad, 2005; Trimble Navigation Limited, 2009). Uncorrected data was differentially postprocessed against the closest public basestation in Deland, Florida (CORS96), approximately 8 kilometers from the mapped area (integrity index = 94.7), using Pathfinder Office. The integrity index is a grading system by Trimble that monitors basestations used for differential processing and rates a basestation on its reliability, accuracy, and precision (Trimble Navigation Limited, 2004). Additionally, the integrity index value for a basestation is adjusted in consideration of the proximity of the basestation to where the data was collected by the rover unit. Trimble recommends postprocessing against a basestation with an integrity index of 80 or higher that is within 200 kilometers of the site (Trimble Navigation Limited, 2009). It is important for the basestation to be in close proximity (a maximum of 200 km) of the roving receiver, as this will allow the receiver and basestation to collect data from the same satellite constellation and produce less error during differential postprocessing (Bolstad, 2005). Processed data were then exported into ArcGIS 10 for analysis in ArcMap. The GIS software used for this research was the latest version of ESRI ArcGIS, version 10 and included the use of ArcCatalog and ArcMap.

| 🖪 GI               | PS Pathfinder Office - [M                   | ap]  |               |   |           |                |                 |
|--------------------|---|--|---------------|---|-----------|----------------|-----------------|
| \$e !              | <u>Eile E</u> dit <u>V</u> iew <u>D</u> ata | <u>U</u> tilities <u>O</u> ptions <u>W</u> indow <u>H</u> elp  |               |   |           |                |                 |
| 2                  | 🗉   🍜   🔈 🚟   🚳                             | +? 🖂 🖹 🖿 🥔   🖑 🔍   🔍 🔍 🔞   | Accuracy_data | 189 GB  |           |                |                 |
| ٠                  | Differential Correc                         | tion Wizard  |               | Select Base Provider                                |           |                | 22              |
| <b>\$</b> 2<br>(0) | *   | Base Data  |               | Provider  | L2 0      | Distance       | Integrity Index |
|                    |   | CORS, DELAND (DLND), FLORIDA   | Select        | & CORS, DELAND (DLND), FLORIDA                      | ×         | 8 km           | 94.70           |
| ri T               | *   | C Folder Search  |               | CORS, ORMOND BEACH (ORMD), FL                       | ORIDA ×   | 37 km          | 92.61           |
| бÐ                 |   | G:\AAFS Poster\Data\Scene_Ant\Base   | Select        | CORS, LEESBURG (LEES), FLORIDA                      | × .       | 51 km          | 94.02           |
|                    |   | C. Prowers   |               | CORS, PLATAKA (PLTK), FLORIDA                       | а, ×<br>* | 78 km<br>78 km | 93.60           |
|                    |   | S DIOWSE   | Province      | CORS, CAPE CANAVERAL 5 (CCV5), 1                    | FLORIDA × | 100 km         | 93.49           |
|                    |   |  | DIDWSG        | & CORS, CAPE CANAVERAL 6 (CCV6), 1                  | FLORIDA × | 100 km         | 93.47 🛫         |
| *                  |   | C Use reference Position Use reference position from base files Use reference position from base provider CORS, DELAND (DLND), FLORIDA | Select        | Show Base Providers of Type: Base Provider New Copy | All types | ▼<br>Delete    | Update List     |
|                    | *   | Confirm base data and position before processing   |               | Help  |           | ОК             | Cancel          |
|                    |   | < <u>B</u> ack Next > Cancel   | Help          |   |           |                |                 |

Figure 9- Screenshot of differential postprocessing using Pathfinder Office

A geodatabase was created for each scenario to organize the collected data (Figure 10). This was accomplished by creating shapefiles from the exported data and grouping these shapefiles into feature datasets. A shapefile is a filetype used in GIS that is a non-topological digital storage format used to store the geometric location of features on a map and includes collected attribute information. Additionally, the filetype allows the easy projection change from one coordinate system to another without losing substantial positional information (Bolstad, 2005). For comparison purposes, coordinate data of the survey markers were also imported into the geodatabase as a shapefile using Microsoft Excel and the XY data tool in ArcGIS.



Figure 10- Screenshot of geodatabase for accuracy data in ArcCatalog

These methods (Figure 11) were first used for the 50-second collection time data and subsequently used for the 100-second collection time data. The 50-second and 100-second were then analyzed to determine accuracy of the collected points to the survey markers.



Figure 11- Flowchart of data planning, collection, processing, and analysis methods

#### Calculating Accuracy

Per Sando et al. (2005), the following formula was used to determine the accuracy of the collected points:

Accuracy = 
$$\sqrt{(x_i - x_0)^2 + (y_i - y_0)^2}$$

Where  $x_i$  is the collected horizontal coordinate,  $x_0$  is the known horizontal coordinate,  $y_i$  is the collected vertical coordinate, and  $y_0$  is the known vertical coordinate. The accuracy was calculated by exporting attribute data into Microsoft Excel version 10 and using the formula tool. Accuracy data were then imported into GIS as attribute information, along with additional statistical analyses discussed later. To ensure correct accuracy calculations, the accuracy of each point was cross-checked using the measuring tool in ArcGIS. On all occasions, the calculated accuracy and the distance measurement in GIS were equal to the nearest hundredth centimeter.

#### **Results**

## Survey Marker Accuracy

The accuracy for each survey marker and collection time was calculated using the aforementioned formula and descriptive statistics (mean, standard deviation, range, and 95% confidence interval) were determined using SPSS Version 20 for comparison purposes (Table 2). The mean accuracy for the tree-covered survey marker (VOLV119) was 42.97 cm and 41.34 cm for the 50-second and 100-second collection times, respectively. Additionally, the mean

accuracy for the survey marker near a tall structure (90501006) was 39.20 cm and 37.30 cm for the 50-second and 100-second collection times, respectively.

The two survey markers in open areas (90501007 and 90501004) showed similar results for both collection times. Survey marker 90501007 demonstrated a accuracy mean of 9.59 cm for 100-second collection time and 11.55 cm for 50-second collection time. Correspondingly, survey marker 90501004 displayed a mean accuracy of 9.51 cm for 100-second collection time and 11.48 cm for 50-second collection time. Therefore, the data collected at survey markers in open areas for both collection times were more accurate than the data collected in obstructed environments, with a mean accuracy of approximately 12.0 cm for 50-second collection time and 10.0 cm for 100-second collection time.

| Survey<br>marker | Environment                | Mean<br>(cm) | Standard<br>deviation (cm) | Range (cm)     | 95% confidence<br>interval (cm) |  |  |
|------------------|----------------------------|--------------|----------------------------|----------------|---------------------------------|--|--|
|                  | 50-second collection time  |              |                            |                |                                 |  |  |
| VOLV119          | Tree cover                 | 42.97        | 2.27                       | 39.57 to 46.32 | 42.08 to 43.86                  |  |  |
| 90501006         | Structure                  | 39.20        | 1.80                       | 34.91 to 42.00 | 38.50 to 39.91                  |  |  |
| 90501007         | Open                       | 11.55        | 1.65                       | 9.13 to 13.38  | 10.89 to 12.21                  |  |  |
| 90501004         | Open                       | 11.48        | 1.75                       | 7.98 to 14.79  | 10.80 to 12.16                  |  |  |
|                  | 100-second collection time |              |                            |                |                                 |  |  |
| VOLV119          | Tree cover                 | 41.34        | 2.87                       | 37.08 to 46.24 | 40.22 to 42.47                  |  |  |
| 90501006         | Structure                  | 37.30        | 1.95                       | 34.85 to 42.62 | 36.54 to 38.06                  |  |  |
| 90501007         | Open                       | 9.59         | 1.82                       | 7.36 to 12.84  | 8.86 to 10.32                   |  |  |
| 90501004         | Open                       | 9.51         | 2.25                       | 5.30 to 13.87  | 8.63 to 10.39                   |  |  |

Table 2- Summary of the accuracy results of 50-second and 100-second collection times

Furthermore, the survey marker under tree cover was found to have the highest standard deviation, and, thus, demonstrated the most variance of the collected points for both collection

times (Table 3). Table 3 shows both the mean accuracy for each survey marker using 50-second and 100-second collection times and the difference between these values, demonstrating that, for all environments, the 100-second collection time was slightly more accurate by approximately 2.0 cm consistently (Table 3). Additionally, Table 4 demonstrates the mean northing and eastings for the survey markers using both collection times, showing that the collected northings were constantly more accurate than the eastings for both collection times and all survey markers.

*Table 3- Average error and difference between collection time error of collected points to survey markers* 

| Survey<br>marker | Environment | Mean<br>50-second<br>error (cm) | Mean<br>100-second<br>error (cm) | Mean error<br>difference<br>(cm) |
|------------------|-------------|---------------------------------|----------------------------------|----------------------------------|
| VOLV119          | Tree cover  | 42.97                           | 41.34                            | 1.63                             |
| 90501006         | Structure   | 39.20                           | 37.30                            | 1.90                             |
| 90501007         | Open        | 11.55                           | 9.59                             | 1.96                             |
| 90501004         | Open        | 11.48                           | 9.51                             | 1.97                             |

Table 4- Mean error of northings and eastings for collected points of survey markers

|                  |             | 50-second        |                 | 100-second       |                 |
|------------------|-------------|------------------|-----------------|------------------|-----------------|
| Survey<br>marker | Environment | Northing<br>(cm) | Easting<br>(cm) | Northing<br>(cm) | Easting<br>(cm) |
| VOLV119          | Tree cover  | 23.51            | 35.92           | 22.04            | 34.90           |
| 90501006         | Structure   | 21.26            | 32.88           | 20.53            | 30.10           |
| 90501007         | Open        | 6.37             | 9.52            | 5.33             | 7.79            |
| 90501004         | Open        | 6.40             | 9.45            | 5.49             | 7.56            |

The results of an independent samples t-test of mean accuracies for the collection times are shown in Table 5. The null hypothesis states that the accuracy of the GPS unit was not
significantly affected by the data collection time. The results from this test show that for all survey markers, there was a significant difference in accuracy between 50- and 100-second collection times (VOLV119 p = .034, 90501006 p = .001, 90501007 p = .000, 90501004 p = .001). These results support the hypothesis that the accuracy of the DGPS unit was significantly increased by 100-second collection time, compared to 50-second collection time in all environments.

| Survey<br>marker | Collection<br>time | Mean<br>(cm) | s.d. (cm) | t      | df | р    |
|------------------|--------------------|--------------|-----------|--------|----|------|
| VOLV119          | 50 s               | 42.97        | 2.27      | _ 2.18 | 19 | 034  |
| (Tree cover)     | 100 s              | 41.34        | 2.87      | 2.10   | 40 | .034 |
| 90501006         | 50 s               | 39.20        | 1.80      | 2 5 2  | 10 | 001  |
| (Structure)      | 100 s              | 37.30        | 1.95      | - 3.32 | 40 | .001 |
| 90501007         | 50 s               | 11.55        | 1.65      | 2.02   | 16 | 000  |
| (Open)           | 100 s              | 9.59         | 1.82      | - 3.83 | 40 | .000 |
| 90501004         | 50 s               | 11.48        | 1.75      | - 2 47 | 10 | 001  |
| (Open)           | 100 s              | 9.51         | 2.25      | - 3.47 | 4ð | .001 |

Table 5- Results of independent samples t-test for both collection times and survey markers

When considering data collected in open areas, there was an approximate 20% accuracy increase using 100-second data collection when compared to 50-second data collection (Table 6). Table 6 shows the results of the independent samples t-test conducted between the different collection times for both survey markers in open areas. The analysis indicates that the different open areas did not yield significantly different GPS coordinates for both 50- and 100-second collection times (50-second p = .884, 100-second p = .893), suggesting that the DGPS unit produced consistent results during data collection of the open areas for both collection times.

| Collection |          |          |     |    |      |
|------------|----------|----------|-----|----|------|
| time       | 90501007 | 90501004 | t   | df | р    |
| 50-second  | 11.55    | 11.48    | 146 | 47 | .884 |
| 100-second | 9.59     | 9.51     | 136 | 47 | .893 |
| % change   | 20.7%    | 20.4%    |     |    |      |

 Table 6- Independent samples t-test of mean error (cm) for 50-second and 100-second collection times and mean percentage changes between collection times for open areas

#### **Distance Measurements**

The actual maximum length of each long bone was compared to the measurements between collected points in ArcGIS for both 50-second and 100-second collection times. Figure 12 illustrates the points collected at the proximal and distal aspects of the long bones in the field for both collection times in an open area. Lines were added in ArcGIS to illustrate the orientation of the long bones. Overall, the 100-second collection time was consistently more accurate than the 50-second collection time when collected points were compared to actual lengths (Table 7). The range of the positional error was 0.1 cm to 2.8 cm for the 100-second collection time and .5 cm to 5.5 cm for the 50-second collection time (Table 7). Long bones with a maximum length greater than 25 cm demonstrated collected points that were closer to the actual maximum length. Additionally, the orientation of long bones with a maximum length greater than 25 cm demonstrated correct orientation and less varied orientation between collection times when compared shorter long bones (Figure 12).



Figure 12- Map of long bone measurements for both collection times

|         | Actual max  | GIS         | GIS max      |             | rence        |
|---------|-------------|-------------|--------------|-------------|--------------|
|         | length (cm) | lengt       | h (cm)       | ( <b>c</b>  | <b>m</b> )   |
|         |             | <u>50 s</u> | <u>100 s</u> | <u>50 s</u> | <u>100 s</u> |
| Humerus | 31.0        | 35.5        | 31.8         | 4.5         | 0.8          |
| Radius  | 23.0        | 21.4        | 25.2         | 1.6         | 2.2          |
| Ulna    | 25.0        | 28.5        | 27.8         | 3.5         | 2.8          |
| Femur   | 42.0        | 47.5        | 44.4         | 5.5         | 2.4          |
| Tibia   | 32.0        | 34.4        | 33.4         | 2.4         | 1.4          |
| Fibula  | 35.0        | 37.1        | 34.3         | 2.1         | 0.7          |
| Os coxa | 22.0        | 21.5        | 22.1         | 0.5         | 0.1          |
|         |             | Mean di     | fference     | 2.9         | 1.5          |

 

 Table 7- Comparison of actual long bone length and lengths between collected points measured in GIS for 50-second and 100-second collection times

Positional data was also collected for known distances (0 cm, 5 cm, 10 cm, 20 cm, 25 cm, and 30 cm) and compared in ArcGIS using the measurement tool. Figure 13 illustrates the collected points at the marked distances with a line symbolizing the measuring tape used in the field to mark the distances. Like the long bone measurements, the 100-second collection time was consistently more accurate than the 50-second collection time, but not by more than 2 cm (Table 8). The range of the positional error was .6 cm to 2.3 cm for the 100-second collection time and 1.1 cm to 2.6 cm for the 50-second collection time (Table 8). Additionally, the error decreased as the distances increased.



*Figure 13- Map of distance measurements for both collection times with tape measure reference and marked intervals* 

Table 8- Comparison of known lengths and lengths of collected points measured in GIS for 50-<br/>second and 100-second collection times

|       | GIS len     | igth (cm)    | Differe     | ence (cm)    |
|-------|-------------|--------------|-------------|--------------|
|       | <u>50 s</u> | <u>100 s</u> | <u>50 s</u> | <u>100 s</u> |
| 5 cm  | 7.5         | 7.3          | 2.5         | 2.3          |
| 10 cm | 12.6        | 11.9         | 2.6         | 1.9          |
| 15 cm | 17.6        | 16.2         | 2.6         | 1.2          |
| 20 cm | 22.2        | 21.4         | 2.2         | 1.4          |
| 25 cm | 26.4        | 25.9         | 1.4         | 0.9          |
| 30 cm | 31.1        | 30.6         | 1.1         | 0.6          |

#### Discussion

Similar results among these environments with increased error from older DGPS models can be found in an earlier study by Sando et al. (2005). The accuracy of this DGPS unit in obstructed areas, such as areas under tree-cover and near tall structures, may not be sufficient for mapping skeletal dispersals because of the high level of error. However, open areas produced consistently accurate positional error data for both collection times for two different unobstructed areas and may be considered for mapping purposes.

In 2001, Graettinger et al. reported that observed accuracies in their study on GPS receiver accuracy were significantly higher than the accuracies reported by the manufacturers in the information provided for their product. The DGPS unit used for this study, however, was consistent with the manufacturer's reported accuracy of 10 to 30 cm. Perhaps because accuracy, along with ease of use, of DGPS units is a concern for most consumers, DGPS retailers, such as Trimble, are conducting more in-depth accuracy determination for newer DGPS units.

Furthermore, several factors not considered in previous studies (Listi et al., 2007 and Spradley et al., 2011) were investigated in this chapter, such as data collection methods and accuracy determination in different environments. By considering data collection methods of long bones and clustered skeletal elements, it was determined that long bones less than 25 cm in length should be measured using 2 points and clustered skeletal elements more than 25 cm apart should be measured as separate features. Additionally, the effect of the environment on the accuracy of DGPS units was also considered by determining the error at known survey points in obstructed environments in addition to open areas.

## Survey Marker Accuracy

The accuracy calculated from the survey markers in open areas for both collection times, demonstrating that the DGPS used for this research produces relatively consistent results at different locations and on different days when in the same environment. When considering the placement of the collected points, the points tended to cluster northwest of the survey marker for all environments and both collection times (Figures 14 and 15). Additionally, the northings (vertical positions) were less accurate than the eastings (horizontal positions), greatly increasing the calculated error. Spradley et al. also found this to be the case for their point data and attributed this trend to be a "well-known limitation of GPS technology" (Spradley et al., 2011:7); however, no literature could be found concerning this trend.



Figure 14- Composite map of collected points and survey markers for 50-second collection time in Deland, FL



Figure 15- Composite map of collected points and survey markers for 100-second collection time in Deland, FL

The independent samples t-test of the survey markers at different collection times demonstrates that the 100-second collection time yielded significantly different accuracy values than the 50-second collection times. This is expected, as the position of the feature collected is an average of each position collected at 1-second intervals for the duration of the collection time. Therefore, if time permits, point data should be collected for a minimum of 100 seconds to ensure increased accuracy.

Furthermore, the use of a rangepole and tripod allowed increased precision of the desired point. In cases where a rangepole is not used (*i.e.* Listi et al., 2007 and Spradley et al., 2011), the unit is placed over the desired point at an approximate height when collecting positional information. This method introduces additional error and can provide inconsistent results. Additionally, the inclusion of a level on the rangepole reinforces the exact location of the desired point, while the tripod holds the unit in place until moved. Finally, the use of an antenna allowed easier access to satellites by extending the height of the unit, increasing the satellite geometry necessary to collect accurate positions and keep the data collector's body from obstructing the satellite signals.

#### **Distance Measurements**

Positional information collected at known intervals demonstrated that long bones less than 25 cm in length should be collected with one point (*i.e.* scapulae, ribs, vertebrae, etc.), as bones longer than 25 cm can demonstrate the orientation of the element by using two points; thus, it is recommended that bones longer than 25 cm be collected with two points (*i.e.* crania, long bones, etc.). Predetermined points should be assigned to bones less than 25 cm in length and should be consistent throughout data collection. For example, it is recommended that scapulae be measured at the glenoid fossa and all vertebrae should be measured at the anterior aspect of the body. Furthermore, it is recommended to map bones as individual features when skeletal elements are at least 25 cm apart, and to map clusters of 2 or more bones that are less than 25 cm apart as one feature. The user may then include information in the "Notes" data entry describing the skeletal elements comprising the cluster.

As seen with the survey marker accuracy, 100-second data collection time was found to be slightly more accurate with a mean difference of 1.5 cm compared to 2.9 cm when comparing long bone lengths. If time permits, it is best to measure all points with a 100-second collection time, rather than 50-second collection time. However, all points should be recorded consistently with the same collection time, whether it is 50-second, 100-second, or any other time allotment.

## **Conclusions**

This chapter presents a quantification of the influence of different factors likely to affect the accuracy of a DGPS receiver used in the collection of positional data. The accuracy of the DGPS unit was determined in 3 different environments: an open area, a tree-covered area, and an area near a tall structure. Two different collections times, 50-second and 100-second, were conducted within the differing environments. When comparing the collection times, it was determined that the 100-second collection time was slightly more accurate (approximately 20%) than the 50-second collection time. Thus, if time permits, point data should be collected at 100second time intervals to insure accurate positional information. However, the difference found between the collection times is minimal when considering the error introduced by the environment in obstructed environments. Additionally, it was determined in this research that the Trimble GeoExplorer 2008 Series DGPS receiver was greatly affected by obstructions within the environment. This unit was accurate to approximately 11.52 cm in open areas, 42.97 cm in areas under tree-cover, and 39.20 cm in areas near a tall structure with a 50-second collection time and 9.55 cm in open areas, 41.34 cm in areas under tree-cover, and 37.30 cm in areas near a tall structure with a 100second collection time.

Further research is necessary to determine the influence of additional factors to the accuracy of DGPS units such as level of cloud cover, degree of tree-cover, direction of the receiver, time of day, PDOP, and additional collection times. In addition to postprocessing, real-time differential processing should also be investigated. Furthermore, error levels should be determined for additional DGPS units, as only a single DGPS unit was considered in this study.

With the inevitable creation of innovative DGPS technology, increased accuracy of DGPS receivers is an endeavor that should be expected in years to come (Lechner and Baumann, 2000). Studies conducted 7 years ago, show error levels of 1.7 m for DGPS units in open environments (Sando et al., 2005), clearly showing the rapid pace at which DGPS technology is developing when compared to the decimeter accuracy determined from this project. It is, thus, the responsibility of the researcher to evaluate the use of these new technologies in the field within different environments and conditions before applying them in forensic situations.

The following chapter will apply the determined accuracy of the Trimble GeoExplorer 2008 Series DGPS unit calculated in this chapter to simulated scenarios within differing environments and levels of scatter. The assembled scenarios will be analyzed in ArcGIS to determine the applicability of mapping these scenarios using this DGPS unit.

# CHAPTER THREE: MAPPING SIMULATED SKELETAL DISPERSALS USING A DGPS UNIT

# **Introduction**

Until recently, GPS units have been used only to collect basic GPS coordinates, such as datums, rather than the specific location of remains within archaeological sites (Napton and Greathouse, 2009). Surveying methods using DGPS units have been conducted archaeologically for several years. DGPS units have been used to survey both large and small archaeological sites all over the world. An ancient road network in Armana, Egypt with a total of 70 roads has been mapped using a DGPS unit (Fenwick, 2001). Several times surveys have been conducted using a DGPS unit to create digital elevation models (DEM) of archaeological sites (Chapman and Van Nort, 2001; Fenwick, 2001). Collier et al. (1995) utilized a DGPS to construct a large triangle irregular network (TIN) for spatial analysis at an archaeological project in Langstone Harbor, England. Also, digital terrain models (DTM) have been created using GPS data (Capra et al., 2002). Additionally, DGPS data collected at archaeological sites can be used not only for surveying purposes but also in several kinds of spatial analyses such as the relationship between elevation, landscape, feature locations, artifact dispersal patterns.

Listi et al. (2003) presented the first research concerning GPS as a utility in mapping skeletal dispersals in a poster presentation at the American Academy of Forensic Sciences. It was determined through this preliminary study that the use of a GPS unit with beacon receiver was only a valuable tool in scene mapping for pinpointing the location of the entire scene or a datum rather than mapping individual skeletal elements.

As discussed in the previous chapter, limited research has been conducted concerning the utility of a DGPS unit in mapping skeletal dispersals. Other than Listi et al.'s (2003) preliminary research, two studies have been published that utilize a DGPS in mapping human remains Listi et al. (2007) and Spradley et al. (2011). However, though Spradley et al. (2011) utilize a DGPS for scene mapping, their research was primarily focused on the analysis of scavenging patterns from vultures on a human cadaver and not the development of a methodology concerning mapping using a DGPS.

In both studies, the type of dispersal (*i.e.* wide scatter versus tight scatter) was not a consideration by the authors. Additionally, both environments in these studies were mixed environments with skeletal elements scattered in both open and obstructed environments. Most importantly, the error values determined by Listi et al. (2007) were calculated from a survey marker in an open environment and were applied to open and wooded environments. Moreover, the determined accuracy using 206-second collection time was inappropriately applied to data collected in various environments at 100-second collection times.

Furthermore, the single simulated scenario constructed by Listi et al. consisted of only 8 features and the skeletal dispersal assessed by Spradley et al. (2011) included only one scenario. Thus, further research with multiple environmental scenarios and dispersal levels must be conducted to accurately assess the practicality of using these enhanced DGPS units in mapping skeletal dispersals.

#### Purpose

The purpose of this chapter is to determine the applicability and practicality of utilizing a DGPS unit in mapping skeletal dispersals. Environments such as open areas, areas under treecover and areas near a tall structure will be considered. In addition, the distribution of the skeleton, such as widely scattered, tightly scattered, and relatively articulated dispersals, will also be considered. The calculated accuracy of the DGPS in the different environments from the previous chapter will be applied to these scenarios as well. Because the DGPS is a relatively new technology that has yet to be comprehensively utilized in the mapping of human remains, different aspects of this utility, such as data collection time, data collected on different days, proximity of features, feature collection, postprocessing methods and attribute data collection, will also be demonstrated.

## Differential Global Positioning System and Scene Mapping

In situations where scattered remains are extensively dispersed over a large area or topographically varied area, standard mapping techniques can be a difficult task (Listi et al., 2007; Napton and Greathouse, 2009). Differential GPS receivers can be easily moved to each skeletal element over a large area, without introducing additional error as a result of longdistance measuring. Furthermore, GPS geospatial data can also be integrated into a GIS which allows the user to analyze and effectively display the mapped scene (Lowe and Burns, 1998; Gao, 2002; Spencer et al., 2003; El-Rabbany, 2006; Dupras et al., 2011). Additionally, DGPS software allows the recordation of attribute data for features through preset data dictionaries, such as bone type and side that can later be accessed in a GIS using an attribute table (Trimble Navigation Limited, 2009). The user may then label the map in GIS with this information for presentation purposes. Furthermore, distance between points can be easily calculated by using a measuring tool (Wheatley and Gillings, 2002). These features may be useful in a court setting where the distance between bones and scene features can be easily determined while testifying, and an inventory of the remains or associated evidence can be referenced.

#### Materials and Methods

As in the previous chapter, the differential GPS used for this research was a Trimble GeoExplorer 2008 Series GeoXH handheld differential GPS receiver with Zephyr antenna. The receiver uses a field computer powered by Microsoft Windows Version 6 operating system and Terrasync software. The receiver uses both H-star and EVEREST multipath technology to provide heightened accuracy after postprocessing using the internal antenna. The addition of the external Zephyr antennae provides better locational recordation with 10 cm to 30 cm accuracy when data is differentially postprocessed (Trimble Navigation Limited, 2009).

# Scenarios

Scenarios were constructed to depict various levels of skeletal dispersals that may be encountered in real-life situations. The following levels of dispersals were considered: wide scatters, tight scatters, and relatively articulated scatters (Table 9). Additionally, the following environments were considered: open areas, tree-covered areas, and areas near tall structures (Table 9).

The environments in this research were chosen to represent three types of outdoor environments that dumped human remains are found. Manhein et al. (2008) found that human remains are dumped in both open and wooded environments or within sight of a structure. Furthermore, the different levels of dispersals were chosen to represent dispersal scenarios of human remains in outdoor settings. Skeletal dispersals have been known to range from relatively articulated skeletons to skeletal elements dispersed over hundreds of meters in rural areas (Manhein et al., 2008).

 Table 9- Summary of types of dispersals and environments considered for the simulated scenarios

| Type of Dispersal               | Environment         |
|---------------------------------|---------------------|
| Wide Scatter                    | Open area           |
| Tight scatter                   | Tree-covered area   |
| Relatively articulated skeleton | Area near structure |

The simulated scenarios were determined by combining each environment with each type of dispersal for a total of nine scenarios (Table 10). Point data were collected with different 50-second and 100-second collection times for all scenarios. Also, offsets and increased productivity settings were implemented in obstructed environments.

| Scenario # | Environment         | Type of dispersal               | Variables considered  |
|------------|---------------------|---------------------------------|---|
| 1          | Open area           | Wide scatter                    | <ul><li>Collection time</li><li>Postprocessing</li><li>Different days</li></ul> |
| 2          | Tree-covered area   | Wide scatter                    | <ul><li>Collection time</li><li>Offsets</li><li>Productivity settings</li></ul> |
| 3          | Open area           | Tight scatter                   | Collection time   |
| 4          | Tree-covered area   | Tight scatter                   | <ul><li>Collection time</li><li>Offsets</li><li>Productivity settings</li></ul> |
| 5          | Open area           | Relatively articulated skeleton | • Collection time   |
| 6          | Tree-covered area   | Relatively articulated skeleton | <ul><li>Collection time</li><li>Offsets</li><li>Productivity settings</li></ul> |
| 7          | Area near structure | Wide scatter                    | <ul><li>Collection time</li><li>Offsets</li><li>Productivity settings</li></ul> |
| 8          | Area near structure | Tight scatter                   | <ul><li>Collection time</li><li>Offsets</li><li>Productivity settings</li></ul> |
| 9          | Area near structure | Relatively articulated skeleton | <ul><li>Collection time</li><li>Offsets</li><li>Productivity settings</li></ul> |

Table 10- Summary table of the simulated scenarios with environment, type of dispersal, and variables considered during data collection

Three areas were chosen to represent the predetermined scenario environments on the University of Central Florida campus (Figure 16). The open area was a cleared area in the University of Central Florida Arboretum. The tree-covered area was also an area in the Arboretum but was densely covered with trees. Finally, the scenarios near a tall structure were conducted in an urban area on the University of Central Florida's campus which was located on the south aspect of Howard Philips Hall.



Figure 16- Aerial image of the scenario locations on the University of Central Florida campus

# Data Collection

Prior to the day of data collection, planning almanac software (available through Trimble) was consulted to determine the best time for data collection. This software provides information including the satellite position data, DOP data, and elevation data on specific days (Figure 17). The best time for data collection was determined by considering the greatest number of satellites, with at least 4 satellites being the most desirable, and the least PDOP value, with values less than 2 being the most desirable (Johnson and Barton, 2004; Bolstad, 2005). Data collection was then conducted during this time period if weather permitted. See Appendix B for planning information for each day of data collection.



Figure 17- Screenshot of planning using Trimble Planning Software

In addition to planning the best time of day for data collection the orientation of the DGPS receiver was also considered. Vertical orientation of the GPS receiver has been found to significantly influence accuracy, with vertical orientation of the receiver yielding coordinates that are more accurate when compared to horizontal orientation (Sando et al., 2005). Data were collected in Universal Transverse Mercator (UTM), Zone 17 North, with the WGS 1984 datum. The rangepole served as the anchor during data collection. The end of the rangepole was positioned at predetermined point adjacent to the skeletal element on the ground and was then

leveled using the dot level on the tripod (Figure 18). The rangepole remained stationary throughout the data collection time.



Figure 18- Image of rangepole placement during data collection with labeled components

Point data were collected using the batch method, which is the average of the point data in 1-second intervals. For example, a batch reading for a point collected at 1-second intervals for 50 seconds would yield the average of 50 points collected at that location. The collection times chosen for this research were chosen in accordance with the 100-second collection time used in previous research (Listi et al., 2007) with the addition of the more efficient 50-second collection

time for comparison purposes.

The following information was recorded during point data collection:

- Date
- Time
- Northing
- Easting
- Max HDOP
- Max PDOP
- Correction Type
- Receiver Type
- Filtered and unfiltered positions
- Feature Name (*i.e.* bone)
- Data dictionary used
- Filename

Point data were collected at each point of interest in 1-second intervals for 50 seconds and 100 seconds. Fifty-second and 100-second data were collected on the same day, consecutively.

The distance analysis conducted in Chapter 2 was implemented during data collection by collecting data for bones with a maximum length of less than 25 cm as a single point, and with a maximum length exceeding 25 cm as two points at opposite aspects (*i.e.* proximal and distal ends) (Table 11). Furthermore, skeletal elements clustered within 25 cm of each other were recorded as a single feature.

|                  | Number    |  |
|------------------|-----------|--|
| Skeletal element | of Points | Description of points collected                      |
| Cronium          | 2         | If oriented sideways: anterior and posterior aspects |
| Crainuin         | 2         | If oriented longways: superior and inferior aspects  |
| Mandible         | 1         | Anterior aspect                                      |
| Vertebrae        | 1         | Anterior aspect of the body                          |
| Sternum          | 2         | Superior and inferior aspect                         |
| Ribs             | 1         | Medial aspect of head                                |
| Scapulae         | 1         | Lateral aspect (glenoid fossa)                       |
| Clavicle         | 1         | Anterior aspect of midshaft                          |
| Os coxa          | 2         | Superior and inferior aspects                        |
| Humerus          | 2         | Proximal and distal aspects                          |
| Radius           | 2         | Proximal and distal aspects                          |
| Ulna             | 2         | Proximal and distal aspects                          |
| Carpal           | 1         | Distal aspect  |
| Metacarpal       | 1         | Distal aspect  |
| Manual phalanx   | 1         | Distal aspect  |
| Articulated hand | 2         | Proximal and distal aspects                          |
| Femur            | 2         | Proximal and distal aspects                          |
| Patella          | 1         | Distal aspect  |
| Tibia            | 2         | Proximal and distal aspects                          |
| Fibula           | 2         | Proximal and distal aspects                          |
| Tarsal           | 1         | Distal aspect  |
| Metatarsal       | 1         | Distal aspect  |
| Pedal phalanx    | 1         | Distal aspect  |
| Articulated foot | 2         | Proximal and distal aspects                          |

Table 11- Description of points and number of points collected on bones and number of pointscollected for each skeletal element

For the scenarios set in obstructed environments (Scenarios 2, 4, 6, 7, 8, and 9), offsets and lowered precision settings were implemented when the satellite geometry was too low for data collection. As discussed in Chapter 2, good satellite geometry is important for accurate data collection and is accomplished by an ideal number and position of satellites overhead. Mid-price DGPS units, such as the GeoXH used for this research, have a slider bar that allows the user to favor productivity instead of precision (Figure 19). Finer precision requires a lower PDOP and, thus, highly favorable satellite geometry (Trimble Navigation Limited, 2009). It is customary when working in areas with interference to increase the productivity, which degrades the precision (Trimble Navigation Limited, 2009). The precision of the unit was lowered until the satellite geometry allowed data to be collected for obstructed skeletal elements and the level of precision was recorded for all skeletal elements that were collected with modified precision.



Figure 19- Productivity versus precision feature in Terrasync

Additionally, basic offsets were applied for skeletal elements in areas that were too obstructed for data collection after employing the productivity feature. The DGPS unit was held stationary in an area with good satellite geometry while the bearing and distance were collected and entered into the unit (Figure 20). When collecting a point using an offset, the DGPS unit collects the point data of the DGPS in the unobstructed area, the user then enters the bearing and distance into the information of the feature (Figure 20). During postprocessing, the software takes into account this offset and determines the coordinates of the feature according to the offset information provided by the user (Trimble Navigation Limited, 2009).



Figure 20- Images of offset use in the field, with (A) bearing measurement and (B) distance measurement



Figure 21- Demonstration of distance and bearing data collection for simple offsets using a DGPS unit

Data were collected using a predefined data dictionary with bone type, bone side, aspect, collection time, and notes (Figure 22). These attributes were later exported into ArcGIS with the point data for analysis and map creation.



Figure 22- Screenshot of point collection with data dictionary in DGPS unit in Terrasync

# Data Processing

After point data were collected in the field, unprocessed data were transferred to a desktop computer using ActiveSync. This data was then imported into Pathfinder Office for differential postprocessing (Figure 23). Postprocessing capabilities refers to the ability of the GPS receiver to store GPS system data in a format that can be used to compute differential corrections of the location data using corrections recorded at a reference receiver to improve locational accuracy (Bolstad, 2005). Uncorrected data were collected in the field and then

differentially postprocessed against the closest public basestation in Deland, Florida (CORS96), approximately 51 miles from the mapped area (integrity index = 94.14), using Pathfinder Office.

The integrity index is a grading system by Trimble that monitors basestations used for differential processing and rates a basestation on its reliability, accuracy, and precision (Trimble Navigation Limited, 2004). Additionally, the integrity index value for a basestation is adjusted in consideration of the proximity of the basestation to where the data was collected by the rover unit. Trimble recommends postprocessing against a basestation with an integrity index of 80 or higher and that is within 200 kilometers of the site (Trimble Navigation Limited, 2009). It is important for the basestation to be in close proximity (a maximum of 200 km) of the roving receiver, as this will allow the receiver and basestation to collect data from the same satellite constellation and produce less error during differential postprocessing (Bolstad, 2005). Processed data were then exported into ArcGIS for analysis in ArcMap. The GIS software used for this research was version 10 of ESRI ArcGIS and included the use of ArcCatalog and ArcMap.



Figure 23- Screenshot of differential postprocessing with basestation selection in ArcCatalog

A geodatabase was created for each scenario to organize the collected data (Figure 24). This was accomplished by creating shapefiles from the exported data and grouping these shapefiles into feature datasets. A shapefile is a filetype used in GIS that is a non-topological digital storage format used to store the geometric location of features on a map and includes collected attribute information (Bolstad 2005; Conolly and Lake, 2006). Additionally, the filetype allows the easy projection change from one coordinate system to another without losing substantial positional information (Bolstad, 2005). For comparison purposes, coordinate data of the survey markers were also imported into the geodatabase as a shapefile using Microsoft Excel and the XY data tool in ArcGIS.



Figure 24- Screenshot of the geodatabase created for the scenarios

These methods were employed for all of the scenarios on different days. The scenarios were then analyzed to determine the practicality of utilizing a DGPS unit in mapping skeletal dispersals.

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|           |               | Provider             |                    |             |                    |                             | L2 G   | Distance       | Integrity Index |        |
| n III     |               | CORS                 | TITUSVIL           | E (COKO)    | FLORIDA            |                             | ×      | 40 km          | ?               |        |
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|           |               | CORS.                | CAPE CAI           | AVERAL 5    | (CCV5), FL         | ORIDA                       | Ŷ      | 65 km          | 93.94           |        |
|           |               | CORS                 | LEESBUR            | G (LEES), F | LORIDA             |                             | ×      | 66 km          | 93.83           |        |
| <u>Ľø</u> |               | 🙇 CORS               | ORMOND             | BEACH (O    | RMD), FLOF         | RIDA                        | ×      | 78 km          | 92.09           |        |
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Figure 25- Screenshot of basestation selection during postprocessing in Pathfinder Office

Unlike Scenarios 2 to 9, Scenario 1 underwent additional postprocessing to ascertain the influence of single and multiple basestations for differential correction, along with different basestations at multiple distances from the area of data collection. Scenario 1 was processed using the Deland basestation (DLND), Cape Canaveral basestation (CCV6), and Leesburg basestation (LEES) (Table 12). This scenario was also processed using multiple basestations (DLND, CCV6, and LEES) (Figure 26) and was compared to differential processing of a single basestation (DLND).

Table 12- Basestations used during postprocessing of Scenario 1 with location, distance, and integrity index

| Basestation | Location           | Distance | <b>Integrity Index</b> |
|-------------|--------------------|----------|------------------------|
| DLND        | Deland, FL         | 51       | 80.97                  |
| CCV6        | Cape Canaveral, FL | 65       | 84.98                  |
| LEES        | Leesburg, FL       | 66       | 81.20                  |



Figure 26- Screenshot of postprocessing with multiple basestations in Pathfinder Office

Processed data were then exported into ArcGIS 10 for analysis in ArcMap. The GIS software used for this research was version 10 of ESRI ArcGIS and included the use of ArcCatalog and ArcMap.

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Figure 27- Screenshot of export from Pathfinder Office to ArcGIS

## Generating Maps and Integrating Accuracy Data

The mean accuracy of each environment (open area, area under tree-cover, and area near a tall structure) determined in Chapter 2 was applied to each scenario using the buffer tool in ArcMap. A buffer shapefile was created using the buffer tool by inputting the point shapefile and setting the radius to 11.52 cm, 42.97 cm, and 39.20 cm for open areas, tree-covered areas, and areas near a tall structure for 50-second collection times, respectively. A buffer was also created for points collected using a 100-second collection time with radii of 9.53 cm, 41.34 cm, and 37.30 cm for open areas, tree-covered areas, and areas near a tall structure, respectively. The radius for the open area was determined as the mean of the error from both survey markers in open areas for both collection times. By adding this shapefile, one is able to take into account

the error of the DGPS for the collected points visually for the scenarios. The individual buffers for each feature were dissolved (Figure28A) to show the overall error for clustered elements and to prevent confusion from interpreting overlapping buffers such as in Figure 28B.



Figure 28- Image from Scenario 4 showing undissolved (A) and dissolved (B) buffers created in ArcMap

## <u>Results</u>

Each scenario is displayed separately to independently illustrate the differences of dispersal type and environment with the addition of error buffers. Overall maps for each scenario and collection time with error buffers are displayed in Appendix C.

#### Scenario 1

Scenario 1 consisted of a wide skeletal dispersal in an open area and was collected using 50-second and 100-second collection times (Figure 29). This scenario was also postprocessed using three different basestations, in addition to being postprocessed against a group of three basestations. Furthermore, the skeletal elements in this scenario were flagged and collected on two different days using the same data collection methods.



Figure 29- Map of 100-second and 50-second collection times with accuracy buffers of Scenario 1 using ArcMap

Point data were compared in ArcGIS to ascertain orientation and length differences between postprocessed data against different basestations and postprocessed data against multiple basestations for both collection times (Figure 30). The orientation of the long bones was generally maintained for all postprocessed data collected using a 100-seond time collection, while the orientation of the long bones using the 50-second collection time was not as consistent. This trend was also demonstrated when considering the actual length of the long bones, with the data collected using the 100-second collection time collecting a maximum length consistent with the actual maximum length of the long bones. The basestation with the greatest distance from Scenario 1, LEES, produced the most inconsistent orientation and the greatest difference from the actual maximum length of the long bones. This is to be expected, as an increase of distance decreases the reliability of a basestation (Bolstad, 2005). All processed point data, however, fell within error buffers for this environment determined in the previous chapter.



*Figure 30- Composite image created using ArcMap of DLND basestation against CCV6 (A), LEES (B), and multiple basestations (C) with actual orientation of long bones in the field* 

Point data collected on different days for Scenario 1 were postprocessed against the DLND basestation and compared in ArcGIS (Figure 31). The general orientation of the long bones were maintained with both collection times for Day 2 but contrasted slightly from the
orientation of the long bones for Day 1. Also, the 100-second collection time recorded maximum lengths closer to the actual maximum lengths of the long bones for both days when compared to the 50-second collection time. The data collected on Day 2 fell within the error buffers determined in the previous chapter for this environment using both collection times.



Figure 31- Map of Scenario 1 for 100-second and 50-second collection time on different days using ArcMap

#### Scenario 2

Scenario 2 consisted of a wide skeletal dispersal in a tree-covered area using 50-second and 100-second collection times. Productivity settings were increased for features in locations that were too obstructed for standard data collection. Offsets were implemented for skeletal elements when locational data could not be obtained using the highest productivity setting. This scenario was differentially corrected against the DLND basestation during postprocessing.

The influence of obstructions on locational data is clearly illustrated in this scenario. The long bones did not maintain maximum length or orientation between the different collection

times. Figure 32 shows the change in orientation of the os coxae and the differences in maximum length of the cranium. Additionally, Figure 32 illustrates the problematic overlapping of the error buffers because of the increased error radius calculated for obstructed environments.



Figure 32- Map of 100-second and 50-second collection times with accuracy buffers of Scenario 2 using ArcMap

Scenario 3 consisted of a tight skeletal dispersal in an open area using 50-second and 100-second collection times. This scenario was differentially corrected against the DLND basestation during postprocessing. Like the wide scatter in an open area (Scenario 1), increased collection time for point data collected in Scenario 3 maintained the relative orientation and maximum length of the long bones (Figure 33). Also, the decreased radius of the error buffer and dispersal type caused minimal overlapping of the features, resulting in an accurate depiction of the dispersal.



Figure 33- Map of 100-second and 50-second collection times with accuracy buffers for cenario 3 using ArcMap

Scenario 4 consisted of a tight skeletal dispersal in a tree-covered area using 50-second and 100-second collection times. Productivity settings were increased for features in locations that were too obstructed for standard data collection. Offsets were implemented for skeletal elements when locational data could not be obtained using the highest productivity setting. This scenario was differentially corrected against the DLND basestation during postprocessing. Scenario 4 produced similar results to Scenario 2, the wide scatter in a tree-covered area, with increased collection time for point data not maintaining the maximum length and orientation of most of the long bones for both collection times (Figure 34). Figure 34 also shows a complete change of direction for the orientation of the right os coxa which is the os coxa near the lower part of the image. Also, the overlap of the clustered features is problematic because of the increased buffer radius for this type of dispersal.



Figure 34- Map of 100-second and 50-second collection times with accuracy buffers of Scenario 4 using ArcMap

Scenario 5 consisted of a relatively articulated skeleton in an open area using 50-second and 100-second collection times. This scenario was differentially corrected against the DLND basestation during postprocessing. Increased collection time for point data collected in this scenario shows preservation of the general orientation and maximum length of the long bones (Figure 35). Some single features, however, did not maintain an exact position but fell within the error buffer. Furthermore, increased overlapping of the error buffers when compared to the other scenarios in open areas (Scenarios 1 and 3) was noted for this type of dispersal.



Figure 35- Map of 100-second and 50-second collection times with accuracy buffers for Scenario 5 using ArcMap

Scenario 6 consisted of a relatively articulated skeleton in a tree-covered area using 50second and 100-second collection times. Productivity settings were increased for features in locations that were too obstructed for standard data collection. Offsets were implemented for skeletal elements when locational data could not be obtained using the highest productivity setting. This scenario was differentially corrected against the DLND basestation during postprocessing. Interestingly, increased collection time for point data collected in Scenario 6 maintained the orientation and maximum length of most long bones (Figure 36), unlike the previous scenarios in tree-covered areas (Scenarios 2 and 4). This may have been due to exceptional satellite geometry obtained during data collection. However, the close proximity of the features and increased buffer radius resulted in excessive overlap of the collected features.



Figure 36- Map of 100-second and 50-second collection times with accuracy buffers for cenario 6 using ArcMap

Scenario 7 consisted of a wide skeletal dispersal in an area near a tall structure using 50second and 100-second collection times. Productivity settings were increased for features in locations that were too obstructed for standard data collection. Offsets were implemented for skeletal elements when locational data could not be obtained using the highest productivity setting. This scenario was differentially corrected against the DLND basestation during postprocessing.

Increased collection time in Scenario 7 slightly changed the orientation for most of the long bones, but maintained the general direction of the element (Figure 37). Additionally, the maximum lengths of the long bones were maintained for both collection times in this environment. However, considerable overlap of error buffers for this type of dispersal was noted.



Figure 37- Map of 100-second and 50-second collection times with accuracy buffers for Scenario 7 using ArcMap

Scenario 8 consisted of a tight skeletal dispersal in an area near a tall structure using 50second and 100-second collection times. Productivity settings were increased for features in locations that were too obstructed for standard data collection. Offsets were implemented for skeletal elements when locational data could not be obtained using the highest productivity setting. This scenario was differentially corrected against the DLND basestation during postprocessing.

Increased collection time for point data collected in Scenario 8 demonstrated a noteworthy difference in the maximum lengths for all long bones collected in this scenario (Figure 38). The general orientation of the long bones, however, was maintained. This contrasts to the point data collected in Scenario 7 (wide scatter near a tall structure), where the maximum length of the long bones was maintained and the orientation of the long bones was slightly effected.



Figure 38- Map of 100-second and 50-second collection times with accuracy buffers for Scenario 8 using ArcMap

Scenario 9 consisted of a relatively articulated skeleton in an area near a tall structure using 50-second and 100-second collection times. Productivity settings were increased for features in locations that were too obstructed for standard data collection. Offsets were implemented for skeletal elements when locational data could not be obtained using the highest productivity setting. This scenario was differentially corrected against the DLND basestation during postprocessing.

Like Scenario 8, the orientation of the long bones was not considerably affected with increased collection time; however, the maximum lengths of the long bones were influenced by the obstructed environment (Figure 39). A difference of maximum length for a majority of long bones was demonstrated. Additionally, considerable overlap of the collected features for this type of dispersal was noted.



Figure 39- Map of 100-second and 50-second collection times with accuracy buffers for cenario 9 using ArcMap

#### Discussion

The point data collected for the scenarios in open areas (Scenarios 1, 3 and, 5) demonstrated a consistent maintenance of long bone orientation and maximum length (Table 13). Furthermore, the overlapping of features in open areas was limited because of the decreased radius of the error buffers. This lack of overlap was demonstrated in all types of dispersals for open environments.

Scenarios in obstructed environments, however, did not produce favorable results when compared to point data collected in scenarios with open areas (Table 13). Preservation of orientation and maximum length of long bones varied between dispersal and environment. The point data collected in the area near a tall structure consistently maintained the general orientation of the long bones but did not maintain the maximum length of the long bones. Additionally, the increased radius of the error buffers for both obstructed environments resulted in considerable overlap of clustered features.

| Scenario | Scenario type  | Maximum long         | Long bone               | Buffer overlap       |
|----------|--|----------------------|-------------------------|----------------------|
| #        |  | bone length          | orientation             |                      |
| 1        | Open wide scatter                                      | Maintained           | Maintained              | Minimal<br>overlap   |
| 2        | Tree-covered wide scatter                              | Not maintained       | Not maintained          | Overlap              |
| 3        | Open tight scatter                                     | Maintained           | Maintained              | Minimal<br>overlap   |
| 4        | Tree-covered tight scatter                             | Not maintained       | Not maintained          | Overlap              |
| 5        | Open relatively articulated skeleton                   | Maintained           | Maintained              | Minimal<br>overlap   |
| 6        | Tree-covered<br>relatively<br>articulated skeleton     | Maintained           | Maintained              | Excessive overlap    |
| 7        | Wide scatter near a structure                          | Generally maintained | Maintained              | Overlap              |
| 8        | Tight scatter near a structure                         | Generally maintained | Not maintained          | Overlap              |
| 9        | Relatively<br>articulated skeleton<br>near a structure | Not maintained       | Generally<br>maintained | Excessive<br>Overlap |

Table 13- Summary of results for maximum long bone length, long bone orientation, and bufferoverlap of Scenarios 1-9

Furthermore, the implementation of decreased productivity settings was necessary in order to gain good satellite geometry for data collection. This resulted in a severely increased data collection time in the field, as it is necessary to increase the productivity and wait for satellite location to be gained. If good satellite geometry is not obtained with the first level increase of productivity, the productivity must be increased again and the user must wait for the satellite location to be gained. This step is repeated until good satellite geometry can be obtained for sufficient locational data to be collected. If the feature was heavily obstructed and good satellite geometry could not be obtained, the use of an offset was implemented. Offsets greatly increased data collection time, as two measurements, the bearing angle and distance measurement, must be collected in addition to moving the DGPS unit to an open area where location information can be collected. However, if several elements are located in a heavily obstructed area, the DGPS may remain stationary while the bearing angle and distance measurement is collected for each feature.

## **Conclusions**

Mapping dispersed remains and associated evidence in the field can be challenging because of long distances and varied environments in which human remains are found. The purpose of this research was to determine if a DGPS unit was a reliable option for mapping skeletal dispersals. It is concluded that mid-price DGPS units are a viable option for mapping skeletal dispersals in open environments. Scenarios mapped in open environments produced consistent results for all types of skeletal dispersals and maintained the orientation and maximum length of long bones. Also, the decreased error determined from the previous chapter did not result in significant overlap of clustered elements.

Conversely, this research also demonstrated that DGPS units do not provide the accuracy and consistency necessary to properly map skeletal dispersals in obstructed environments. The orientation and maximum lengths of the skeletal elements were constantly inaccurate in these environments, demonstrating that error introduced from obstructions severely affected collection of location data. Also, the error determined in the previous chapter resulted in constant overlap of proximate skeletal elements. Thus, the collection of locational information for datums at

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obstructed sites may be utilized to tie in positional information of the entire site but should not be used to separately map skeletal elements.

This is the first research developing a methodology for determining the accuracy of a DGPS unit in different environments using survey markers and applying the calculated accuracy to simulated scenarios in different environments. Further research, however, is necessary to ascertain the applicability of DGPS units in mapping skeletal dispersals. Most importantly, it is necessary for additional DGPS units to be tested and compared to the Trimble GeoExplorer 2008 Series receiver. Also, the accuracy of DGPS units in additional obstructed environments (*i.e.* areas near mountains) and mixed environments should be determined. Further, real-time differential postprocessing should also be considered in future research.

## CHAPTER FOUR: INTEGRATING DGPS DATA INTO A GIS

## **Introduction**

GIS systems are equipped with an abundance of tools that are valuable in analyzing and mapping DGPS data. However, the literature is limited concerning how DGPS data used for mapping skeletal dispersals can be analyzed in a GIS. Listi et al. (2007) mentions the benefits of using a GIS for analyzing and mapping skeletal dispersals but does not explain how data may be integrated into a GIS or what tools may be beneficial in analysis and mapping human remains. Manhein et al. (2006) utilized a GIS on a larger-scale and conducted spatial analysis of several dumpsites; however, this study used a single coordinate for an entire site and did not consider spatial analysis of the elements within a site. It was not until Spradley et al.'s study (2011) concerning spatial patterning of vulture scavenging, that DGPS data from a single skeletal dispersal was analyzed and mapped in a GIS.

This chapter will discuss the benefits of integrating DGPS data into a GIS for analysis and generating maps. Additionally, this chapter will summarize the findings of the research conducted and discuss the desirable conditions in which a DGPS unit may be utilized in mapping skeletal dispersals. Most importantly, guidelines and best-of-use practices when employing a DGPS unit in mapping skeletal dispersals will be provided.

#### **Geographical Information Systems**

In order to understand how the integration of DGPS data into a GIS can be useful in mapping human remains, it is important to first understand the mechanics of GIS. A GIS is best described as a set of tools used for analyzing spatial data (Napton and Greathouse, 2009). It is software that has the capability to display spatially referenced data, analyze data in a spatial geodatabase, and generate maps. Most importantly, a GIS allows spatial information to be converted into useful data, stored, analyzed, and then displayed (Clarke, 1995; Napton and Greathouse, 2009).

GIS software stores attribute data in a database and then references this data to geographical data that has been collected or is readily available in the software (ESRI, 1990). GIS software may then be used to manipulate and analyze the data, in addition to generating maps for presentation. GIS was developed primarily from computer mapping systems and remote sensing technology in the 1990's. It has since been utilized to represent and model geographic and archaeological data (Napton and Greathouse, 2009).

### Analysis of DGPS data in GIS

Primarily, the use of a GIS in archaeology has been to demonstrate the relation of past social systems in relation to their environment in addition to quantitative site location analysis (Conolly and Lake, 2006). As mentioned earlier, research concerning the use of a GIS in smaller scenes, such as skeletal dispersals, however, has only been considered by a single study. Spradley et al. (2011) utilized DGPS data in a GIS to analyze the spatial distribution of skeletal elements from vulture scavenging in a controlled study, but did not develop a methodology concerning the utility of the DGPS unit in mapping. Though several tools were used to analyze DGPS data in GIS, the study focused on dispersal patterns, rather than the data collection and accuracy of the DGPS.

However, the preliminary research conducted by Spradley et al. (2011) for spatial analysis of a dispersal using GIS demonstrates that GIS analysis can serve as a beneficial tool in analyzing a scene. This study shows that descriptive spatial analytical and spatial statistics measures can be utilized to summarize special patterns in the displacement of remains. Furthermore, the calculation of a Mean Center, a point that identifies the geographic center of a set of locations, of the remains can be determined using GIS software to track changes in the distribution of a certain feature. Additionally, the extension and direction of scattered remains can be analyzed using Directional Distribution, which measures the directional trend of a set of features, and Standard Distance, which measures the degree of dispersion of features around the determined Mean Center.

Though Spradley et al. (2011) utilized sophisticated spatial analyses in their study, simpler tools available in a GIS can be used as well. For example, the measuring tool measures the distance between points for analysis (Figure 40). Area may also be measured by drawing a polygon with the cursor and entire features may be measured by selecting the feature, which is particularly useful in a courtroom setting if the witness is asked for a measurement between features to show context of a scene.

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Figure 40- Screenshot of measuring tool in ArcMap from Scenario 1

The buffer tool is convenient when the user is looking to illustrate the possible error of the DGPS unit, demonstrated in Chapter 3. This tool creates buffer polygons around input features to a specified distance. Also, an optional dissolve can be performed to combine overlapping features. In addition to illustrating error of a unit, this tool can be used to show the diameter for a group of clustered skeletal elements.

As mentioned previously in this chapter, preliminary research has been conducted concerning spatial analysis at both small-scale and large-scale levels. There exists an abundance of spatial analysis tools available in GIS that have yet to be explored. For example, Cluster and Outlier Analysis or Hot Spot Analysis (Figure 41), identifies statistically significant hot spots, cold spots, and spatial outliers and has the potential to be beneficial in dispersals with more than a single individual to aid in the determination of primary deposition location. Additionally, the use of Spatial Autocorrelation (Figure 41), which measures autocorrelation based on feature locations and attribute values, could be beneficial in assessing the dispersal patterns of bone type at a scene to better understand the distribution of the elements since deposition.

| Input Feature Class                            | Input Feature Class                            |
|--|--|
| Input Field                                    | Input Field                                    |
| Output Feature Class                           | Generate Report (optional)                     |
| Conceptualization of Spatial Relationships     | Conceptualization of Spatial Relationships     |
| FIXED_DISTANCE_BAND                            | INVERSE_DISTANCE -                             |
| Distance Method                                | Distance Method                                |
| EUCLIDEAN DISTANCE                             | EUCLIDEAN_DISTANCE -                           |
| Standardization                                | Standardization                                |
| NONE   | NONE   |
| Distance Band or Threshold Distance (optional) | Distance Band or Threshold Distance (optional) |
| Self Potential Field (optional)                | Weights Matrix File (optional)                 |
| Weights Matrix File (optional)                 |  |
| OK Cancel Environments Show Help >>            | OK Cancel Environments Show Help >>            |

Figure 41- Screenshots of data input for Hot Spot Analysis and Spatial Autocorrelation tools in ArcGIS

## Creating Maps Using DGPS data in GIS

In addition to analysis in a GIS, an advantage of integrating DGPS data in a GIS is to create detailed maps of a scene. GIS has several features that may be utilized to clearly illustrate the scene as it was in the field and highlight aspects of the scene that were not recognized in the field. One example is the one-click addition of a basemap available in ESRI ArcMap version 10, which allows the user to add an aerial image to the DGPS data. The addition of a basemap brings context to the scene by adding streets, trees, and other landmarks. Basemaps add a broader perspective of the scene for wide scatters, showing possible dispersal patterns in accordance with surrounding features, such as changes in vegetation or streets. Aerial images also are appealing in a courtroom setting when it is necessary for jurors to understand the overall location of a scene.

Several types of basemaps can be added to georeferenced data in ArcMaps version 10 such as aerial imagery, road and street maps, topographic maps, and terrain maps (Figure 42). However, the resolution of the aerial imagery is limited and is not appropriate for all dispersal scenarios. When considering the data collected for this research, one can retain contextual information in an aerial photo with wide scatters of at least 30 meters when compared to tight scatters (Figure 43). Furthermore, aerial basemaps can be useful in all outdoor environments such as tree-covered and urban areas (Figure 44). The spatial relationship of skeletal elements to manmade features, such as buildings or sidewalks, and natural features, such as trees, can be easily illustrated. Additionally, basemaps with roads can be used for dispersals in urban areas to add contextual information (Figure 45). Most importantly, basemaps are beneficial in illustrating the location of an entire scene for any level of skeletal dispersal with any type of environment using a variety of basemaps.



Figure 42- Screenshot of basemap feature in ArcMap



Figure 43- Composite image from ArcMap of aerial basemaps of a simulated wide scatter (left) and a simulated tight scatter (right).



Figure 44- Composite image from ArcMap of aerial basemaps of simulated skeletal dispersals in a tree-covered area (left) and an urban area (right)



Figure 45- Composite image from ArcMap of a roads basemap of a skeletal dispersal in an urban area, both zoomed out (left) and zoomed in (right)

In addition to adding a basemap layer to DGPS data, elements can be easily labeled or highlighted by using drawing tools. Users can label elements in a variety of ways using various colors to highlight certain features. Data may be grouped according to attribute data collected in the field such has bone type, completeness, or side. Legends can then be generated by GIS for maps. Shapes can also be added to emphasize features, group features together, or connect features via lines. Text can be added by labeling points with attribute data or can be added to a map manually. Additionally, the callout tool can be used to add information to a location or label a feature (Figure 46). The bookmark tool can save the parameters for an area of interest. This is useful for presentation purposes when the user wishes to zoom in or out of a known area on queue without making multiple maps (Figure 47).



Figure 46- Screenshot of callout tool in ArcMap from Scenario 1



Figure 47- Screenshot of bookmark tool in ArcMap from Scenario 1

Attribute information collected in the field can serve as a preliminary skeletal inventory or valuable cross-reference for a skeletal inventory. Furthermore, attribute data of separate features can be grouped according to bone type, allowing the user to visually catalog the remains that were collected in the field. Also, the time at which positional data is collected in the field is automatically recorded and exported as attribute information for each feature which can aid in maintaining the chain of custody during. This may also be implemented for associated evidence at the scene.

Finally, one of the most beneficial reasons for using a GIS to generate maps of scene is that throughout the map-making process, geospatial information is retained. Georeferenced data will not change from zooming in and out or by adding additional layers for comparison purposes. All of the features, shapes, and layers added to the map will stay to scale and all geospatial will be maintained.

## Guidelines for Data Collection of DGPS data

Before data collection can occur, it must be determined if mapping using a DGPS unit is appropriate for the scene in question. This research determined that a mid-price DGPS unit is not a viable tool in mapping skeletal dispersals in obstructed environments. However, a DGPS unit may be utilized to map skeletal dispersals in open environments when certain practices are implemented for data collection and postprocessing. The use of traditional mapping methods such as a compass survey or baseline may introduce additional error from long distances and obstructions for widely dispersed remains (Listi et al., 2007). Furthermore, a total station may not be a viable mapping method in these situations because of the obstruction of the line of site over long distances which results in the relocation of the transit point to gain an accurate sight line from the total station unit to the stadia point (Listi et al., 2007; Napton and Greathouse, 2009; Dupras et al., 2011). It is in scenarios such as these that a DGPS unit may be a better option because of its portability and ease of use in wide skeletal dispersals, in addition to the geospatial information that can be provided from using a DGPS. However, for situations with a relatively articulated skeleton or skeletal elements in close proximity, a DGPS unit may not be the best mapping option. Moreover, if the remains are located in an obstructed environment, an error range of approximately 40 cm is not appropriate when a more accurate method, such as a baseline, may be employed.

Before a DGPS unit is used for mapping purposes, the accuracy of the unit must be determined by comparing coordinate information to a known survey marker. This may be accomplished by following the practices provided in the Materials and Methods section of Chapter 2. Data concerning survey markers may be obtained from the Department of

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Transportation in the user's area. If the error determined by the DGPS unit is within the accuracy range desired by the user, the DGPS unit may be employed in similar environmental scenarios (*i.e.* open area error applied to dispersals in open areas).

Furthermore, developing a consistent methodology for data collection is crucial for mapping any scene. By following predetermined procedures, there is less of a chance of introducing additional error or not including important information. Before data collection, determine how the point data will be collected for each skeletal element. For example, all long bones may be measured using proximal and distal ends, while short bones, such as vertebrae, will be measured at predetermined aspects, such as the anterior of the body. Furthermore, a rangepole should be used to better pinpoint the exact location of the element that the user is measuring. The addition of a tripod to the rangepole is recommended to keep the DGPS unit straight and level during location acquisition. Per Chapter 2, it is recommended that bones longer than 25 cm would be mapped using two points to illustrate orientation, while bones less than 25 cm should be mapped using a single point. Additionally, it is recommended that clustered elements less than 25 cm apart should be measured as a single element, while clustered elements grouped farther than 25 cm apart should be measured as separate features. Furthermore, when collecting point data for more than one feature, information concerning the type or description of skeletal element should be recorded during data collection. For a full list of points used to collect points of skeletal elements in this research, see Table 11.

After determining how point data will be collected for each type of bone, a data dictionary should be created to include any additional attribute information. It is recommended

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that the following information be included in order to ensure a complete inventory of the skeletal elements and characteristics:

- Bone type (*i.e.* cranium, mandible, etc.)
- Side
- Completeness (*i.e.* complete, 75%, etc.)
- Aspect (*i.e.* proximal, distal, etc.)
- Collection time
- Environment type
- Date collected
- Time collected
- Notes

It is possible to assign default selections to a field within the data dictionary, such as type of environment, to save time during data collection. Also, time of data collection and date of data collection can be generated by the software instead of being entered manually. Another advantage of using a data dictionary is that the software will not allow the user to save a feature unless all of the required information is entered, ensuring that crucial information is not missed in the field. Additionally, by including a field for notes in the data dictionary, the user is free to include any additional information concerning the feature. Finally, it is recommended that the data dictionary be as generic as possible, so that it may be applied to several types of scenes without having to create a new data dictionary each time a scene is mapped.

An important part of successful data collection is good satellite geometry, which is the desired number and position of satellites for optimal accuracy during data collection (Bolstad, 2005). To achieve good satellite geometry, it is recommended that the user consult a satellite almanac to determine the best time of day for data collection. Thus, prior to the day of data collection, the best time for data collection can be determined by consulting preplanning software. Planning almanac software is free, available through Trimble or other major DGPS

retailers, and is obtainable up to a year in advance. This software provides information including the satellite position data, DOP data, and elevation data on specific days. It is recommended that data collection be determined by considering the greatest number of satellites, with at least 4 satellites being the most desirable, and the least PDOP value, with values less than 2 being the most desirable (Johnson and Barton, 2004). Time windows fitting these characteristics should be at least 3 hours long to allow for any unforeseen circumstances. If preplanning is not possible, it is recommended that the PDOP value and number of satellites be recorded if poor satellite geometry were to occur, as this may explain an increased error of point data.

After a data collection method has been established and once the best time of day for data collection has been determined, point data may be collected in the field. It is recommended that all predetermined methods be executed consistently throughout the data collection process. First, skeletal elements and evidence should be flagged, so that features are not missed. It is also recommended that the DGPS unit be oriented vertically, so as not to degrade satellite reception (Sando et al., 2005). If possible, one user should conduct the entire survey, so that possible error from differences in data collection methods is not introduced and features are not skipped.

The collection time for point data should be determined in consideration of the number of elements to be mapped and the amount of time available in the field. The collection time should be as long as possible, as increased collection time adds point data that will be averaged for the final coordinate. It is recommended that at least a 100-second collection time be implemented, as this collection time was found to be more accurate than a 50-second collection time; however, if time is a factor a 50-second collection time was found to provide sufficient accuracy in open

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areas. Once the collection time is determined, this collection time should be utilized for all point data.

After positional data has been collected for the objects of interest, collected features should be cross-checked with the flagged skeletal elements and evidence. This may be accomplished by looking at the list of features collected or with a map of the area on the data view screen. The user may also collect features such as trail entrances, datums, and buildings to give context to the site. This may be accomplished by creating lines for roads or polygons for buildings using the DGPS unit or simply with point data.

Once all data has been collected, it is necessary to transfer the data to a computer where the data will be differentially corrected against a nearby basestation. After the data is transferred, the data must be imported into differential correction software such as Pathfinder Office. Generally, there is a short amount of lag time (approximately 1 to 2 hours) in receiving basestation data. When choosing a basestation to differentially correct the DGPS data against, it is crucial to choose the basestation that is within the closest proximity to the scene at which the data was collected. Trimble recommends postprocessing against a basestation with an integrity index of 80 or higher and that is within 200 kilometers of the site (Trimble Navigation Limited, 2009). Once the data has been successfully corrected, data must then be exported as a shapefile for analysis in a GIS. Figure 49 shows a flowchart of the guidelines provided in this chapter.



Figure 48- Flow chart of guidelines for collecting and processing DGPS data for skeletal dispersals

## **Conclusion**

This chapter explored the various options that are available by integrating DGPS data from a scene into a GIS. Several tools may be employed to investigate the spatial distribution of remains and analyze scatter patterns of a skeletal dispersal; however, an abundance of tools available in GIS analysis have not yet been used to assess their usefulness for a scene with human remains. Thus, further research with these tools is necessary to determine their applicability and practicality for analyzing skeletal dispersals.

In addition to spatial analysis of DGPS data in a GIS, this chapter considered the advantages of using a GIS for generating maps of a skeletal dispersal. Several tools such as the addition of a basemap, shapes, text, and colors can be utilized to illustrate the scene as it was in the field. Furthermore, these tools may be used to highlight certain aspects of a scatter while preserving geospatial information for the features collected at the site.

Finally, detailed guidelines concerning the use of a DGPS unit in mapping skeletal dispersals were provided. Because of decreased accuracy from obstructions, scenes in open environments should only be used for mapping. It was found in Chapter 3 that scenes with obstructions such as trees or tall buildings significantly affected the accuracy of the DGPS unit. With the consideration to the environment in which DGPS data will be collected, the use of guidelines provided in this chapter can provide an accurate depiction of dispersed human remains and associated evidence, while maintaining geospatial information and attribute data of the features collected.

This research has served as the first thorough investigation utilizing a DGPS unit and GIS in mapping scattered human remains. As mentioned previously, research concerning the use of

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DGPS equipment and GIS software in scene mapping and analysis is limited. Additional research must be conducted to assess the practicality and applicability of these utilities for mapping skeletal dispersals. Further research using additional DGPS units in other environments is needed to assess the practicality of this utility in additional scenarios. Furthermore, additional research concerning the integration of DGPS data into a GIS must be conducted to explore additional tools that may be valuable in the analysis of scenes with scattered remains for both small-scale and large-scale situations.

# **APPENDIX A: SURVEY MARKERS**
| FDOT D5 PID  | PNC STATION REPORT   | Thursday, Jar   | nuary 11, 2007 2:34:58 P  |
|--|--|---|---|
| 90501004   | PNC WORK GROUP   | SURVET  |   |
| DATA PROVIDED IS FOR INFOR<br>TO THE USER. USER ASSUMES                                    | MATIONAL PURPOSES ONLY. DATA IS<br>ALL RISK AND LIABILITY FROM DATA                                  | SUBJECT TO CHANGE AND RE<br>USAGE UNLESS AUTHORIZED                                 | VISION WITHOUT NOTICE<br>OTHERWISE BY FDOT.                     |
| DUE TO THE USE OF AUTOMATED<br>THIS REPORT MAY BE EXPRESSE<br>QUALITY STATEMENT FOR EACH F | IMPORTATION PROCESSES INTO THE<br>D AT A HIGHER PRECISION THAN ACTI<br>PUBLISHED COMPONENT TO DETERM | PNC DATABASE THE PUBLISH<br>UALLY DETERMINED. PLEASE F<br>INE APPROPRIATE USAGE PRE | ED VALUES AS DEPICTED IN<br>REFER TO THE POSITIONAL<br>CISIONS. |
| HECTOR_NAME 90501004   | EFB_NAME N/A   |   | RE_NAME N/A   |
| GOT_VECTOR   | ,  | GOT   |   |
| PRECISE LATITUDE / LONGITUDE   | DESTROYED  |   | ROXIMATE LATITUDE / LONGITUDE                                   |
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| LONGITUDE 81 18 26.  | .67263 LOST STATIO   |   | GITUDE N/A N/A N/A  |
| LATITUDE / LONGITUDE POSITION  |  |   | UDE / LONGITUDE REFERENCE DATUM                                 |
| PPHNC BY GPS NETWORK OBSE  | RVATIONS   | NAD   | 83 (1990)   |
| LATITUDE / LONGITUDE PPHNC<br>PUBLICATION SOURCE   | 90501. 90501_RPT12.TXT, DAT  | ED 2006.11.28.  |   |
| ELLIPSOID HEIGHT IN US SVY FEE   | T ELLIPSOID HEIGHT IN  | METERS ELLIPSOID H  | EIGHT REFERENCE DATUM   |
| -53.4874   | -16.3030   | NAD 83  |   |
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| SPC - NORTHING US SVY FEET   | SPC - NORTHING METERS  | STATE PLANE PROJECTION SC   | CALE FACTOR   |
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| SPC - EASTING US SVY FEET  | SPC - EASTING METERS   | STATE PLANE ZONE  |   |
| 557925.8759  | 170056.1471  | FLORIDA EAST  |   |
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| ELEVATION N/A  |  |   |   |
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| FDOT D5 PID  | PNC STATION REPORT  | Thursday, January 11, 2007 2:34:58 P   |  |  |  |
|--|---|--|--|--|--|
| 90501004   | PNC WORK GROUP  |  |  |  |  |
| DATA PROVIDED IS FOR INFORMAT<br>TO THE USER. USER ASSUMES ALI   | NONAL PURPOSES ONLY. DATA IS SUBJECT<br>- RISK AND LIABILITY FROM DATA USAGE U  | TO CHANGE AND REVISION WITHOUT NOTICE<br>NLESS AUTHORIZED OTHERWISE BY FDOT.   |  |  |  |
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| GPS OBSERVATION POTENTIAL  | OD  |  |  |  |  |
| STATION VISITATION SAFETY CONSID   | ERATION   |  |  |  |  |
| NO OBSERVED EXTRAORDINARY SA   | AFETY CONSIDERATIONS REPORTED FO  | THIS STATION SITE.   |  |  |  |
| DESCRIPTION OF STATION MONUMEN   | T - DISK / CAP AND STAMPINGS  |  |  |  |  |
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| PPHCN 90501 90501004 FIELD 3<br>AT THE INTERSECTION OF S.R.<br>ALONG S.R. 44 (NEW YORK AVE.) AN<br>OF S.R.44 (NEW YORK AVE.) AN<br>CONCRETE SIDEWALK.  | STATION REPORT, DATED 2005.12.21<br>44 (NEW YORK AVE.) AND S.R. 15A<br>.) FOR 1.0 MILES TO THE STATION (<br>ND CLARA AVE. STATION IS IN THE S | . (ALL DISTANCES ARE APPROXIMATE)<br>(SPRING GARDEN AVE.), PROCEED EASTERLY<br>ON THE RIGHT. STATION IS AT THE INTERSECTION<br>SOUTHEAST QUADRANT OF THE INTERSECTION IN A |  |  |  |
| REFERENCE         OBJECTS:         PPHCN 905         S           S 81 W         11.8 FT.         MAST ARM           S 82 E         23.6 FT.         POWER POI           S 10 W         75.5 FT.         NW-CORNEI           N 00 E         8.2 FT.         BACK OF C   | 501 90501004 FIELD STATION REPOR:<br>STEEL COLUMN<br>LE, WOOD, NO # ,MAG NAIL AND [FDG<br>& OF POST OFFICE BLDG.<br>CONCRETE CURB             | r, DATED 2005.12.21.<br>DT REF.] DISK  |  |  |  |
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| STATION HISTORY  | ASSOCIATED MEDIA FILES AVAILABLE  | ON CD.   |  |  |  |
| 2006.10.25 BY J. HAWKINS OF<br>STATION MONUMENTED: YES. DEG<br>STATION OCCUPIED : YES.<br>NETWORK NAME : 90501 9050100<br>STATION ALIAS: N/A<br>EFB NAME: N/A  | FDOT D5 STAFF, REPORTING ON PPH<br>CEMBER 2005. (FDOT)<br>04  | CN 90501, F.P. NO.4046461.   |  |  |  |
| LATITUDE : ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83 (1990).<br>LONGITUDE: ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83 (1990).<br>ELLIPSOID HEIGHT: ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83.<br>GPS NGVD 29 ELEVATION: NOT ESTABLISHED.<br>GPS NAVD 88 ORTHOMETRIC HEIGHT: ESTABLISHED. PPHNC.<br>LEVELED NGVD 29 ELEVATION: NOT ESTABLISHED.<br>LEVELED NGVD 88 ORTOMETRIC HEIGHT: NOT ESTABLISHED. |   |  |  |  |  |
| NOTE: STATION PHOTOGRAPHY TA   | AKEN  |  |  |  |  |
| NOTE: "XY" ESTABLISHED LATI!<br>NOTE: "H" ESTABLISHED ELLIPS<br>NOTE: "h" ESTABLISHED GPS NA   | TUDE/LONGITUDE IN NETWORK.<br>SOID HEIGHT IN NETWORK. 90501 RP:<br>AVD 88 ORTHOMETRIC HEIGHT IN NETV  | r03.TXT, DATED 2006.11.28.<br>WORK.  |  |  |  |
| FINAL NETWORK ADJUSTMENT REL   | PORT: 90501_RPT12.TXT, DATED 2000   | 5.11.28.   |  |  |  |

FDOT D5 PID

PNC STATION REPORT FDOT D5 LOCATION SURVEY

Thursday, January 11, 2007 2:34:58 P

90501004

DUE TO THE USE OF AUTOMATED IMPORTATION PROCESSES INTO THE PNC DATABASE THE PUBLISHED VALUES AS DEPICTED IN THIS REPORT MAY BE EXPRESSED AT A HIGHER PRECISION THAN ACTUALLY DETERMINED. PLEASE REFER TO THE POSITIONAL QUALITY STATEMENT FOR EACH PUBLISHED COMPONENT TO DETERMINE APPROPRIATE USAGE PRECISIONS.

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| TELD DAPTY.                    | THO                        | KT.              | - TA.11                  |           |             |  |
|--------------------------------|----------------------------|------------------|--------------------------|-----------|-------------|--|
| IELD PARII:                    |                            | KIN,             | <u> -, 144(</u>          | OR, H,    | NEV         | WCOMB  |
| ULL NEIWORK S.                 | TATION NAME                | : 0              |                          | 0100      | )4          | FDOTDS PID: <u>70501004</u>                                      |
| AND HELD CLING                 | OMETER: YES                | -                | NO                       |           | HA          | AND HELD COMPASS: YES NO   |
| THER                           |                            |                  |                          |           |             | 8  |
| .I. MEASURED N                 | WITH: <u>-00</u>           | DIN              | G RUL                    | £         |             |  |
| BSTRUCTIONS OI                 | BSERVED AT                 | H.I. C           | OF 1.790                 | m ABOVE N | ORMAL       | L GROUND AT STATION SITE.  |
| TATION MARK IS                 | s <u>0.00</u>              | METERS           | ABOVE                    | BELOW 📈   | NORM        | MAL GROUND AT STATION SITE.                                      |
| = AZIMUTH AND<br>= ANGLE OF ED | GLE FROM MA<br>LEVATION FR | GNETIC<br>OM THE | NORTH AS D<br>APPARENT H | ETERMINED | BY H<br>THE | HAND COMPASS. FULL CIRCLE SWEEP<br>INSTRUMENT OBSERVATION HEIGHT |
| BEGIN OBSTRUC                  | CTION                      |                  | END OBST                 | RUCTION   |             | OBSTRUCTION DESCRIPTION  |
| Он                             | 15 V                       | то               | 254 н                    | 15        | V           | NO OBSTRUCTION   |
| 254 н                          | 30 V                       | то               | 273 н                    | 30        | v           | TREE LINE  |
| 273 н                          | 15 V                       | то               | 360 H                    | 15        | v           | NO OBSTRUCTION   |
| Н                              | v                          | TO               | H                        | [         | V           | 8  |
| Н                              | v                          | то               | H                        | [         | v           | 10<br>6  |
| H                              | v                          | то               | H                        | [         | v           |  |
| Н                              | v                          | TO               | Н                        | [         | v           |  |
| H                              | v                          | то               | H                        | []        | v           |  |
| H                              | v                          | TO               | <u>ь </u> Н              | ſ         | v           | 4  |
| H                              | v                          | TO               | H                        | ı         | V           |  |
| Н                              | v                          | TO               | H                        | ı         | V           |  |
| H                              | v                          | TO               | H                        | []        | V           |  |
| H                              | v                          | TO               | H                        | I         | v           |  |
| H                              | v                          | TO               | H                        |           | V           |  |
| н                              | v                          | то               | H                        |           | V           | × :  |
| H                              | v                          | то               | H                        |           | v           |  |
| H                              | v                          | то               | H                        |           | V           |  |
|                                |                            |                  |                          |           |             |  |
|                                |                            |                  | C                        | OMMENTS   |             |  |
|                                |                            |                  |                          |           |             |  |
|                                |                            |                  |                          |           |             |  |

| FDOT D5 PID<br>90501006<br>DATA PROVIDED IS FO<br>TO THE USER. USER  | DR INFORMATIONAL PL   | PNC STATION REPO<br>FDOT D5 LOCATIOI<br>PNC WORK GROUP<br>JRPOSES ONLY. DATA<br>D LIABILITY FROM DA | RT<br>N SURVEY<br>IS SUBJECT TO CH<br>TA USAGE UNLESS         | Thursday, April 24, 20<br>ANGE AND REVISION<br>AUTHORIZED OTHER                           | 08 3:21:01 PM<br>WITHOUT NOTICE<br>WISE BY FDOT.   |
|--|---|---|---|---|--|
| DUE TO THE USE OF AUT<br>THIS REPORT MAY BE EX<br>QUALITY STATEMENT FO   | OMATED IMPORTATIO<br>(PRESSED AT A HIGHE<br>R EACH PUBLISHED C                                    | N PROCESSES INTO T<br>R PRECISION THAN AC<br>OMPONENT TO DETER                                      | HE PNC DATABASE<br>CTUALLY DETERMIN<br>RMINE APPROPRIAT       | THE PUBLISHED VAL<br>IED. PLEASE REFER 1<br>E USAGE PRECISIONS                            | UES AS DEPICTED IN<br>TO THE POSITIONAL<br>3.  |
| HECTOR_NAME 90501<br>GOT_VECTOR<br>PRECISE LATITUDE / LOU<br>LATITUDE 29 0<br>LONGITUDE 81 1<br>LATITUDE / LONGITUDE<br>PPHNC BY GPS NETWO | NGITUDE<br>1 41.60855<br>7 57.09887<br>POSITIONAL QUALITY<br>RK OBSERVATIONS                      | EFB_NAME N/A<br>DESTROYE<br>ELE STATIC<br>REVIEWED<br>LOST STAT<br>NOT CONN                         | ED STATION DON DON DON DON DON DON DON DON DON D              | 3WIRE_NAM<br>GOT_LEVEL<br>APPROXIMA<br>LATITUDE<br>LONGITUDE<br>LATITUDE / L<br>NAD 83 (1 | E N/A<br>TE LATITUDE / LONGITUDE<br>N/A N/A N/A<br>N/A N/A N/A<br>ONGITUDE REFERENCE DATUM |
| ELLIPSOID HEIGHT IN US   | SVY FEET  | ELLIPSOID HEIGHT I  | IN METERS   | ELLIPSOID HEIGHT  | REFERENCE DATUM  |
|  |   | IL. GPS NETWOR  | K OBSERVATIONS  | INTED 00  |  |
| ELLIPSOID HEIGHT<br>PUBLICATION<br>SOURCE  | NC 90501. 90501_1   | RPT12.TXT, DATED  | 2006.11.28.   | -   |  |
| SPC - NORTHING US SVY           1706673.3718           SPC - EASTING US SVY F           560551.4301  | FEET         SPC - NOR*           520195.01           FEET         SPC - EAST           170856.41 | THING METERS<br>841<br>ING METERS<br>176  | STATE PLANE PL<br>.99995165<br>STATE PLANE ZO<br>FLORIDA EAST | ROJECTION SCALE FA  | CTOR   |
| STATE PLANE COORDINA   | TE POSITIONAL QUALI   | TY PPHNC BY GPS   | NETWORK OBSER   | VATIONS   |  |
| STATE PLANE<br>PUBLICATION<br>SOURCE   | 90501. 90501_RPT  | 12.TXT, DATED 20  | 06.11.28.   |   |  |
| DIFFERENTIALLY LEVELE<br>ORTHOMETRIC HEIGHT  | DL ORTHOMETRIC  | HEIGHT IN US SVY FE   | ET DLORTHOME  |   | ORTHOMETRIC HEIGHT DATUM   |
| GPS DERIVED<br>ORTHOMETRIC HEIGHT  | GPS ORTHO HEIGH   | HT IN US SVY FEET   | GPS ORTHO   | HEIGHT IN METERS  | NAVD 88  |
| ORTHOMETRIC HEIGHT<br>POSITIONAL QUALITY   | APPROXIMATE BY C  | GPS NETWORK OBSEF   | RVATIONS  |   |  |
| ORTHOMETRIC HEIGHT<br>PUBLICATION SOURCE   | PPHNC 90501. 905  | 501_RPT12.TXT, DA   | ATED 2006.11.28   | ł .   |  |
| ELEVATION  | ELEVATION IN US SVY   | FEET EL   | EVATION IN METER  | S ELEVA   | ATION DATUM  |
| ELEVATION<br>POSITIONAL QUALITY  | N/A   |   |   |   |  |
| ELEVATION<br>PUBLICATION SOURCE  | N/A   |   |   |   |  |

| FDOT D5 PID<br>90501006   | PNC STATION REPORT<br>FDOT D5 LOCATION SURVEY<br>PNC WORK GROUP  | Thursday, April 24, 2008 3:21:01 PM   |  |  |  |
|---|--|---|--|--|--|
| DATA PROVIDED IS FOR INFORMAT<br>TO THE USER. USER ASSUMES ALL  | IONAL PURPOSES ONLY. DATA IS SUBJECT T<br>RISK AND LIABILITY FROM DATA USAGE UNI   | O CHANGE AND REVISION WITHOUT NOTICE<br>.ESS AUTHORIZED OTHERWISE BY FDOT.                                    |  |  |  |
| DUE TO THE USE OF AUTOMATED IMP<br>THIS REPORT MAY BE EXPRESSED AT<br>QUALITY STATEMENT FOR EACH PUBL   | ORTATION PROCESSES INTO THE PNC DATAE<br>A HIGHER PRECISION THAN ACTUALLY DETE<br>ISHED COMPONENT TO DETERMINE APPROP            | BASE THE PUBLISHED VALUES AS DEPICTED IN<br>RMINED. PLEASE REFER TO THE POSITIONAL<br>RIATE USAGE PRECISIONS. |  |  |  |
| GPS OBSERVATION POTENTIAL   | ac   |   |  |  |  |
| STATION VISITATION SAFETY CONSIDE   | ERATION  |   |  |  |  |
| NO OBSERVED EXTRAORDINARY SA  | FETY CONSIDERATIONS REPORTED FOR   | THIS STATION SITE.  |  |  |  |
| DESCRIPTION OF STATION MONUMEN  | T - DISK / CAP AND STAMPINGS   |   |  |  |  |
| MONUMENT CONSTRUCTION: CONCR<br>MONUMENT CONDITION: GOOD<br>DISK/CAP LOCATION ON MONUMEN<br>DISK/CAP SETTING: DRILL HOLE<br>DISK/CAP CONSTRUCTION: MAG N<br>DISK/CAP OCCUPATION POINT: D<br>DISK/CAP CASTING STAMP: [MAG<br>DISK/CAP HAND STAMPING: [D5P<br>DISK/CAP CONDITION: GOOD<br>DISK/CAP STABILITY: GOOD  | ETE SIDEWALK<br>T: FLUSH ON TOP<br>AND EPOXY<br>AIL AND 1.5" STEEL WASHER<br>IMPLE IN CENTER OF NAIL HEAD<br>]<br>NC] [90501006] |   |  |  |  |
| STATION SITE "TO REACH" DESCRIPTION   | ON   |   |  |  |  |
| PHECN 00501 00501006 FIELD STATION REPORT, DATED 2005.12.21. (ALL DISTANCES ARE APPROXIMATE)<br>AT THE INTERSECTION OF S.R. 44 (NEW YORK AVE.) AND S.R. 15A (SPRING GARDEN AVE.), PROCEED EASTERLY<br>ALONG S.R. 44 (NEW YORK AVE.) FOR 1.5 MILES TO THE STATION ON THE LEFT. STATION IS AT THE INTERSECTION<br>OF S.R. 44 (NEW YORK AVE.) AND AMELIA AVENUE STATION IS IN THE NORTHEAST QUADRANT OF THE INTERSECTION<br>IN A CONCRETE SIDEWALK.<br>REFERENCE OBJECTS: PHECN 90501 90501006 FIELD STATION REPORT, DATED 2005.12.21.<br>N 80 W 4 0 FT FLEOR N AMELIA AVE. N-BOIND LANES MAG NALL AND (FDOT BEE) DISK |  |   |  |  |  |
| S 75 E 13.0 FT. 12" PALM<br>S 20 E 20.8 FT. STEEL MAS<br>N 00 E 86.1 FT. METAL SIG  | TREE, MAG NAIL AND [FDOT REF] DIS<br>T ARM SUPPORT COLUMN<br>N, "NO LEFT TURN"   | K   |  |  |  |
| SURVEY BASELINE STATION PLUS  | SURVEY BASELINE STATION OFFSET US  | SVY FT METERS   |  |  |  |
| N/A   | N/A  |   |  |  |  |
| SOR BASELINE DESCRIPTION PUBLICA  | TION SOURCE  |   |  |  |  |
| N/A   |  |   |  |  |  |
| STATION HISTORY   | ASSOCIATED MEDIA FILES AVAILABLE   | DN CD.  |  |  |  |
| 2006.10.25 BY J. HAWKINS OF<br>STATION MONUMENTED: YES. DEC<br>STATION OCCUPIED : YES.<br>NETWORK NAME : 90501 9050100<br>STATION ALIAS: N/A<br>EFB NAME: N/A   | FDOT D5 STAFF, REPORTING ON PPHCN<br>EMBER 2005. (FDOT)<br>6   | 90501, F.P. NO.4046461.   |  |  |  |
| LATITUDE : ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83 (1990).<br>LONGITUDE: ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83 (1990).<br>ELLIPSOID HEIGHT: ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83.<br>GPS NGVD 29 ELEVATION: NOT ESTABLISHED.<br>GPS NAVD 88 ORTHOMETRIC HEIGHT: ESTABLISHED. PPHNC.<br>LEVELED NGVD 29 ELEVATION: NOT ESTABLISHED.<br>LEVELED NGVD 88 ORTOMETRIC HEIGHT: NOT ESTABLISHED.  |  |   |  |  |  |
| NOTE: STATION PHOTOGRAPHY TA  | KEN  |   |  |  |  |
| NOTE: "XY" ESTABLISHED LATIT<br>NOTE: "H" ESTABLISHED ELLIPS<br>NOTE: "h" ESTABLISHED GPS NA  | UDE/LONGITUDE IN NETWORK.<br>OID HEIGHT IN NETWORK. 90501 RPT0<br>.VD 88 ORTHOMETRIC HEIGHT IN NETWO                             | 3.TXT, DATED 2006.11.27.<br>RK.   |  |  |  |
| FINAL NETWORK ADJUSTMENT REP  | ORT: 90501_RPT12.TXT, DATED 2006.  | 11.28.  |  |  |  |

FDOT D5 PID

PNC STATION REPORT FDOT D5 LOCATION SURVEY Thursday, April 24, 2008 3:21:01 PM

90501006

PNC WORK GROUP

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2008.04.11 BY D. SARACINA OF D5 FDOT STAFF, REPORTING ON FP 4220242. STATION RECOVERED: YES. APRIL 2008. (FDOT) GOOD CONDITION STATION NAME: 90501006

NOTE: STATION RESEARCHED FOR POSSIBLE USE - NON PNC HORIZONTAL. RECOVERY DATA PROVIDED TO LOCHRANE. NOTE: NO FURTHER UPDATE TO RECORD.

| AGE 1 OF   | FLORI                     | DA DEPART                                  | MENT  | C OF<br>STAT: | TRANSPORT     | ATION          | UCTION M | T FI        | VE LOCATION SURVEY SECTION<br>NG REPORT                          |
|--|---------------------------|--|-------|---------------|---------------|----------------|----------|-------------|--|
| Stall party:       J. HAWKIN J. F. TAHIOR, A. ALEWICOME         UUL NETWORK STATION NAME: $OP$ $OS$ $OI$ $OOG$ FDOTDS FID: $2050100$ HAND HELD CLINOMETER:       YES       NO       HAND HELD COMPASS:       YES       NO         ALL MEASURED WITH: $EODTUG$ RULE       NO       HAND HELD COMPASS:       YES       NO         ALL MEASURED WITH: $EODTUG$ RULE       NO       HAND HELD COMPASS:       YES       NO         ALL MEASURED WITH: $EODTUG$ RULE       NO       HAND HELD COMPASS:       YES       NO         ALL MEASURED WITH: $EODTUG$ RULE       NORMAL GROUND AT STATION SITE.       Intronometers       Intronometers       Intronometers       Yes       NO         Ital ASIGNETION BERMETION FROM THE APPARENT HORIZON AT THE INSTRUMENT OBSERVATION HEIGHT       BESTRUCTION DESCRIPTION       OBSTRUCTION DESCRIPTION         DESTRUCTION       END OBSTRUCTION       OBSTRUCTION DESCRIPTION       OBSTRUCTION       DESTRUCTION         SET $A$ $Z$ $Y$ $A$ $Z$ $Y$ $B$ $Z$ $Y$ $A$ $Z$ $Y$ $B$ $Z$ $Y$ $B$  | AGE 1 OF                  |  |       |               |               |                |          |             | DATE: 2005. /Z. Z/   |
| ULL NETWORK STATION NAME:  | IELD PARTY:               | J. HA                                      | WK    | IN            | , F. TA       | 1011           | R, A.,   | NEL         | NCOMB  |
| AND HELD CLINOMETER: YES NO HAND HELD COMPASS: YES NO   THER   A.I. MEASURED WITH: FOLD THG- RULE   BESTRUCTIONS OBSERVED AT H.I. OF /.790 m ABOVE NORMAL GROUND AT STATION SITE.   TATION MARK IS 0.00 METERS ABOVE   | ULL NETWORK               | STATION N                                  | AME : | 0             | 9 05          | 0              | 1_00     | 06          | FDOTD5 PID: 90501006   |
| A.I. MEASURED WITH: $for the form the$ | AND HELD CLI              | NOMETER :                                  | YES   | L             | NO            |                |          | HA          | ND HELD COMPASS: YES 👉 NO _                                      |
| A.I. MEASURED WITH:       FOLDTUG       RULE         DESTRUCTIONS OBSERVED AT H.I. OF       /.790 m ABOVE NORMAL GROUND AT STATION SITE.         TATION MARK IS       0.00 METERS ABOVE  | THER                      |  |       |               |               |                |          |             |  |
| Destructions observed at h.i. of /.792 m above normal ground at station site.         STATION MARK IS 0.00 METERS ABOVE  | .I. MEASURED              | WITH: _                                    | FOL   | DI            | UG R          | ULE            |          |             | 8  |
| STATION MARK IS 0.00       METERS ABOVE BELOWNORMAL GROUND AT STATION SITE.         H = AZIMUTH ANGLE FROM MAGNETIC NORTH AS DETERMINED BY HAND COMPASS. FULL CIRCLE SWEEP         Z = ANGLE OF ELEVATION FROM THE APPARENT HORIZON AT THE INSTRUMENT OBSERVATION HEIGHT         BEGIN OBSTRUCTION       END OBSTRUCTION       OBSTRUCTION DESCRIPTION         BEGIN ODSTRUCTION       END OBSTRUCTION       OBSTRUCTION DESCRIPTION         B H       Z2       V       TO       S       H       Z2       V       BIDG*.         3528       H       /5       V       TO       S       H       Z2       V       BIDG*.         3528       H       /5       V       TO       TS       H       Z2       V       BIDG*.         3528       H       /5       V       TO       TS       H       Z2       V       BIDG*.         3528       H       /5       V       TO       DESTRUCTION       DESTRUCTION         75       H       60       V       TO       Z2       NO       OBSTRUCTION         129       H       15       V       NO       OBSTRUCTION       HEIGHT         H       V       TO       H       V       NO       DESTRUCTION <td>BSTRUCTIONS</td> <td>OBSERVED</td> <td>AT H</td> <td>H.I.</td> <td>OF 1.790</td> <td><u>2</u>m</td> <td>ABOVE NC</td> <td>RMAL</td> <td>GROUND AT STATION SITE.</td>  | BSTRUCTIONS               | OBSERVED                                   | AT H  | H.I.          | OF 1.790      | <u>2</u> m     | ABOVE NC | RMAL        | GROUND AT STATION SITE.  |
| I = AZIMUTH ANGLE FROM MAGNETIC NORTH AS DETERMINED BY HAND COMPASS. FULL CIRCLE SWEEP         I = ANGLE OF ELEVATION FROM THE APPARENT HORIZON AT THE INSTRUMENT OBSERVATION HEIGHT         BEGIN OBSTRUCTION       END OBSTRUCTION       OBSTRUCTION DESCRIPTION         2       H       15       V       TO       Image: Structure         3       H       22       V       TO       Image: Structure       OBSTRUCTION         3       H       22       V       TO       Image: Structure       OBSTRUCTION       OBSTRUCTION         35       H       22       V       TO       Image: Structure       Image: Structure       Image: Structure         75       H       /5       V       TO       Image: Structure       Image: Structure       Image: Structure         75       H       /60       V       TREPLIANT       Image: Structure       Image: Structure         1/29       H       60       V       TREPLIANT       Image: Structure       Image: Structure         1/29       H       60       V       TREPLIANT       Image: Structure       Image: Structure         1/29       H       /5       V       MO       OBSTRUCTION       Image: Structure         H       V       TO  | TATION MARK               | IS 0.00                                    | M     | IETER         | S ABOVE _     | BE             | LOW _    | NORM        | AL GROUND AT STATION SITE.                                       |
| BEGIN OBSTRUCTION       END OBSTRUCTION       OBSTRUCTION       OBSTRUCTION DESCRIPTION         2       H       15       V       TO       3       H       15       V       AU OBSTRUCTION       OBSTRUCTION         8       H       22       V       TO       3       H       22       V       BIDG         35       H       .75       V       TO       .75       H       .15       V       AU OBSTRUCTION         75       H       .15       V       TO       .75       H       .15       V       AU OBSTRUCTION         75       H       .06       V       TO       .129       H       .60       V       TR22LINE         .129       H       .15       V       TO       .44       .45       .40       .40       .40         .129       H       .15       V       TO       .44       .40       .40       .40       .40       .40          H </td <td>= AZIMUTH A<br/>= ANGLE OF</td> <td>NGLE FROM<br/>ELEVATION</td> <td>MAG</td> <td>NETI<br/>M TH</td> <td>C NORTH A</td> <td>S DEI<br/>T HOR</td> <td>ERMINED</td> <td>BY H<br/>THE</td> <td>AND COMPASS. FULL CIRCLE SWEEP.<br/>INSTRUMENT OBSERVATION HEIGHT</td>   | = AZIMUTH A<br>= ANGLE OF | NGLE FROM<br>ELEVATION                     | MAG   | NETI<br>M TH  | C NORTH A     | S DEI<br>T HOR | ERMINED  | BY H<br>THE | AND COMPASS. FULL CIRCLE SWEEP.<br>INSTRUMENT OBSERVATION HEIGHT |
| D       H       15       v       TO       9       H       15       v       NO OBSTRUCTION.         9       H       22       v       TO       35       H       22       v       BlDG         35       H       75       H       15       v       NO OBSTRUCTION.         75       H       00       v       TO       75       H       00       v         75       H       60       v       TREELIANE       v       NO OBSTRUCTION.         129       H       15       v       NO OBSTRUCTION.       NO         H       V       TO       360       H       15       v       NO OBSTRUCTION.         H       V       TO       H       V       NO       OBSTRUCTION.         H       V       TO       H       V       NO       OBSTRUCTION.         H       V       TO       H       V       NO       OBSTRUCTION.         H       V       TO       H       V       NO       NO         H       V       TO       H       V       NO       NO         H       V       TO       H       V  | BEGIN OBSTR               | UCTION                                     |       |               | END O         | BSTRU          | CTION    |             | OBSTRUCTION DESCRIPTION  |
| 8       H       22       V       TO       35       H       22       V       BIDG         352       H       /5       V       TO       75       H       /5       V       MO       OBSTRUCTION/         75       H       60       V       TO       129       H       60       V       TREELINE         129       H       15       V       TO       360       H       15       V       NO       OBSTRUCTION/         H       V       TO       H       V       TO       H       V       NO       OBSTRUCTION/         H       V       TO       H       V       TO       H       V       NO       OBSTRUCTION/         H       V       TO       H       V       NO       OBSTRUCTION/       NO         H       V       TO       H       V       NO       OBSTRUCTION/       NO         H       V       TO       H       V       NO       OBSTRUCTION/       NO       NO         H       V       TO       H       V       V       NO       NO       NO       NO       NO         H       V  | <u></u> н                 | 15   | V     | TO            | - 8           | H              | 15       | v           | NO OBSTRUCTION.  |
| 3522       H       /5       V       TO       75       H       /5       V       NO       OBSTRUCTION         75       H       60       V       TO       129       H       60       V       TREELINE         129       H       15       V       TO       360       H       15       V       NO       OBSTRUCTION         H       V       TO       360       H       15       V       NO       OBSTRUCTION         H       V       TO       H       V       NO       OBSTRUCTION       NO         H       V       TO       H       V       NO       NO <td>8 н</td> <td>22</td> <td>v</td> <td>TO</td> <td>35</td> <td>H</td> <td>22</td> <td>v</td> <td>BIDG</td>  | 8 н                       | 22   | v     | TO            | 35            | H              | 22       | v           | BIDG   |
| 75       H       60       V       TO       129       H       60       V       TREELINE         129       H       15       V       TO       360       H       15       V       NO OBSTRUCTION         H       V       TO       H       V       V       NO OBSTRUCTION         H       V       TO       H       V       V         H       V       TO       H       V   | 3522 н                    | 15   | V     | TO            | 75            | H              | 15       | v           | NO OBSTRUCTION   |
| 129       H       15       V       NO OBSTRUCTION         H       V       TO       H       V         H <td>7<u>5</u> H</td> <td>60</td> <td>v</td> <td>TO</td> <td>129</td> <td>H</td> <td>60</td> <td>V</td> <td>TREELINE</td>   | 7 <u>5</u> H              | 60   | v     | TO            | 129           | H              | 60       | V           | TREELINE   |
| H       V       TO       H       V         H       V </td <td>129 н</td> <td>15</td> <td>V</td> <td>TO</td> <td>360</td> <td>H</td> <td>15</td> <td>V</td> <td>NO OBSTRUCTION</td>   | 129 н                     | 15   | V     | TO            | 360           | H              | 15       | V           | NO OBSTRUCTION   |
| H       V       TO       H       V         H       V </td <td>Н</td> <td><u></u></td> <td>V</td> <td>TO</td> <td>P<u></u></td> <td>H</td> <td></td> <td>V</td> <td></td>   | Н                         | <u></u>                                    | V     | TO            | P <u></u>     | H              |          | V           |  |
| H       V       TO       H       V         H       V </td <td>H</td> <td></td> <td>V</td> <td>TO</td> <td></td> <td>H</td> <td></td> <td>v</td> <td></td>  | H                         |  | V     | TO            |               | H              |          | v           |  |
| H       V       TO       H       V   | Н                         |  | V     | TO            | -             | H              |          | V           |  |
| H      V       TO      H      V  | H                         | •  | V     | TO            | <u> </u>      | H              |          | V           | 5<br>  |
| H       V       TO       H       V   | H                         | e <mark>n anna ha dean anna an</mark><br>R | V     | TO            | -             | H              |          | V           |  |
| H     V     TO     H     V  | H                         |  | V     | TO            |               | H              |          | V           |  |
| H     V     TO     H     V   | H                         |  | V     | TO            | 3 <u></u>     | H              |          | V           |  |
| H     V     TO     H     V   | Н                         | 19   | V     | TO            |               | H              |          | V           |  |
| HV TOHV<br>HV TOHV<br>HV TOHV  | H                         |  | V     | TO            | 5 <u></u>     | H              |          | v           |  |
| HV TOHV  | H                         |  | V     | TO            |               | H              |          | V           |  |
| HV TOHV  | Н                         | 0  | V     | TO            |               | H              |          | V           |  |
|  | Н                         |  | V     | TO            | 3 <del></del> | H              |          | V           | 51.<br>  |
| COMMENTS   |                           |  |       |               |               | COM            | MENTS    |             |  |
|  |                           |  |       |               |               |                |          |             |  |

| FDOT D5 PID  |   | Thursday, January 11, 2007 2:47:55 P  |  |  |  |  |  |
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| 90501007   | PNC WORK GROUP  |   |  |  |  |  |  |
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| DUE TO THE USE OF AUTOMATED IM<br>THIS REPORT MAY BE EXPRESSED A<br>QUALITY STATEMENT FOR EACH PUE | PORTATION PROCESSES INTO THE PNC DATABA<br>T A HIGHER PRECISION THAN ACTUALLY DETER<br>SLISHED COMPONENT TO DETERMINE APPROPR | SE THE PUBLISHED VALUES AS DEPICTED IN<br>MINED. PLEASE REFER TO THE POSITIONAL<br>IATE USAGE PRECISIONS. |  |  |  |  |  |
| HECTOR_NAME 90501007<br>GOT_VECTOR   | EFB_NAME N/A  | 3WIRE_NAME N/A<br>GOT_LEVEL   |  |  |  |  |  |
| PRECISE LATITUDE / LONGITUDE   |   | APPROXIMATE LATITUDE / LONGITUDE  |  |  |  |  |  |
| LATITUDE 29 01 41.08   |   | LATITUDE N/A N/A N/A  |  |  |  |  |  |
| LONGITUDE 81 17 27.14  |   | LONGITUDE N/A N/A N/A   |  |  |  |  |  |
| LATITUDE / LONGITUDE POSITIONAL  |   | LATITUDE / LONGITUDE REFERENCE DATUM  |  |  |  |  |  |
| PPHNC BY GPS NETWORK OBSERV  | ATIONS  | NAD 83 (1990)   |  |  |  |  |  |
| LATITUDE / LONGITUDE PPHNC 90  | )501. 90501_RPT12.TXT, DATED 2006.11  |   |  |  |  |  |  |
| ELLIPSOID HEIGHT IN US SVY FEET  | ELLIPSOID HEIGHT IN METERS  | ELLIPSOID HEIGHT REFERENCE DATUM  |  |  |  |  |  |
| -22.5295   | -6.8670   | NAD 83  |  |  |  |  |  |
| ELLIPSOID HEIGHT POSITIONAL QUAL   | ITY PPHNC BY GPS NETWORK OBSERVATIO   | DNS   |  |  |  |  |  |
| ELLIPSOID HEIGHT<br>PUBLICATION<br>SOURCE  | ELLIPSOID HEIGHT PPHNC 90501. 90501_RPT12.TXT, DATED 2006.11.28.<br>PUBLICATION<br>SOURCE                                     |   |  |  |  |  |  |
| SPC - NORTHING US SVY FEET S   | PC - NORTHING METERS STATE PLANE  | E PROJECTION SCALE FACTOR   |  |  |  |  |  |
| 1706613.3847 5   | 20176.8000 .99995108  |   |  |  |  |  |  |
| SPC - EASTING US SVY FEET S  | PC - EASTING METERS STATE PLANE   | EZONE   |  |  |  |  |  |
| 563210.6324 1  | 71666.9441 FLORIDA EA   | AST   |  |  |  |  |  |
| STATE PLANE COORDINATE POSITIONAL QUALITY PPHNC BY GPS NETWORK OBSERVATIONS                        |   |   |  |  |  |  |  |
| STATE PLANE<br>PUBLICATION<br>SOURCE   | 501_RPT12.TXT, DATED 2006.11.28.  |   |  |  |  |  |  |
| DIFFERENTIALLY LEVELED DL ORTHORTRIC HEIGHT  | IOMETRIC HEIGHT IN US SVY FEET DL ORTHO   | ORTHOMETRIC HEIGHT DATUM  |  |  |  |  |  |
| GPS DERIVED<br>ORTHOMETRIC HEIGHT GPS OR   | THO HEIGHT IN US SVY FEET         GPS ORTH           8         21.0580  | HO HEIGHT IN METERS   |  |  |  |  |  |
| ORTHOMETRIC HEIGHT APPROXIM  | ATE BY GPS NETWORK OBSERVATIONS   |   |  |  |  |  |  |
| ORTHOMETRIC HEIGHT<br>PUBLICATION SOURCE   | 501. 90501_RPT12.TXT, DATED 2006.11.  | .28.  |  |  |  |  |  |
| ELEVATION ELEVATION  | IN US SVY FEET ELEVATION IN MET   | ITERS ELEVATION DATUM   |  |  |  |  |  |
| ELEVATION<br>POSITIONAL QUALITY  |   |   |  |  |  |  |  |
| ELEVATION PUBLICATION SOURCE   |   |   |  |  |  |  |  |

| FDOT D5 PID  | PNC STATION REPORT  | Thursday, January 11, 2007 2:47:55 P   |  |  |  |
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| GPS OBSERVATION POTENTIAL  | DOD   |  |  |  |  |
| STATION VISITATION SAFETY CONSIL   | DERATION  |  |  |  |  |
| NO OBSERVED EXTRAORDINARY S  | AFETY CONSIDERATIONS REPORTED FC  | OR THIS STATION SITE.  |  |  |  |
| DESCRIPTION OF STATION MONUMER   | NT - DISK / CAP AND STAMPINGS   |  |  |  |  |
| MONUMENT CONSTRUCTION: CONC<br>MONUMENT CONDITION: GOOD<br>DISK/CAP LOCATION ON MONUME<br>DISK/CAP SETTING: DRILL HOL<br>DISK/CAP CONSTRUCTION: MAG<br>DISK/CAP OCCUPATION POINT:<br>DISK/CAP CASTING STAMP: [MA<br>DISK/CAP HAND STAMPING: [FD<br>DISK/CAP STABILITY: GOOD<br>DISK/CAP STABILITY: GOOD  | RETE SIDEWALK<br>NT: FLUSH ON TOP<br>E AND EPOXY<br>NAIL AND 1.5" STEEL WASHER<br>DIMPLE IN CENTER OF NAIL HEAD<br>G]<br>OT] [REF]                          |  |  |  |  |
| STATION SITE "TO REACH" DESCRIPT   | ION   |  |  |  |  |
| PPHCN 90501 90501007 FIELD<br>AT THE INTERSECTION OF S.R.<br>ALONG S.R. 44 (NEW YORK AVE<br>INTERESECTION OF S.R. 44 (N<br>SIDEWALK.   | STATION REPORT, DATED 2005.12.21<br>44 (NEW YORK AVE.) AND S.R. 15A<br>.) FOR 2.0 MILES TO THE STATION<br>EW YORK AVE.) AND BOSTON AVENUE                   | 1. (ALL DISTANCES ARE APPROXIMATE)<br>A (SPRING GARDEN AVE.), PROCEED EASTERLY<br>ON THE RIGHT. STATION IS AT THE<br>IN THE SOUTHEAST QUADRANT IN A CONCRETE |  |  |  |
| REFERENCE         OBJECTS:         PPHCN         90           S         67         E         20.2         FT.         10"         PALM           S         28         E         55.7         FT.         NW-CORNE           S         00         W         65.6         FT.         POWER PO           N         06         E         6.6         FT.         BACK OF                                      | 501 90501007 FIELD STATION REPOR<br>TREE, MAG NAIL AND [FDOT REF.]<br>R OF BRICK BLDG. #702 E. NEW YOF<br>LE, WOOD, NO #, MAG NAIL AND [FE<br>CONCRETE CURB | RT, DATED 2005.12.21.<br>DISK<br>RK AVE.<br>DOT REF.] DISK   |  |  |  |
| SURVEY BASELINE STATION PLUS   | SURVEY BASELINE STATION OFFSET  | US SVY FT METERS   |  |  |  |
| N/A  | N/A   |  |  |  |  |
| SOR BASELINE DESCRIPTION PUBLIC  | ATION SOURCE  |  |  |  |  |
| N/A  |   |  |  |  |  |
| 1  |   |  |  |  |  |
| STATION HISTORY  | ASSOCIATED MEDIA FILES AVAILABL   | JE ON CD.  |  |  |  |
| 2006.10.25 BY J. HAWKINS OF<br>STATION MONUMENTED: YES. DE<br>STATION OCCUPIED : YES.<br>NETWORK NAME : 90501 905010<br>STATION ALIAS: N/A<br>EFB NAME: N/A  | FDOT D5 STAFF, REPORTING ON PPH<br>CEMBER 2005. (FDOT)<br>07  | HCN 90501, F.P. NO.4046461.  |  |  |  |
| LATITUDE : ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83 (1990).<br>LONGITUDE: ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83 (1990).<br>ELLIPSOID HEIGHT: ESTABLISHED. PPHNC. GPS OBSERVATION. NAD 83.<br>GPS NGVD 29 ELEVATION: NOT ESTABLISHED.<br>GPS NAVD 88 ORTHOMETRIC HEIGHT: ESTABLISHED. PPHNC.<br>LEVELED NGVD 29 ELEVATION: NOT ESTABLISHED.<br>LEVELED NGVD 88 ORTOMETRIC HEIGHT: NOT ESTABLISHED. |   |  |  |  |  |
| NOTE: STATION PHOTOGRAPHY T.   | AKEN  |  |  |  |  |
| NOTE: "XY" ESTABLISHED LATI<br>NOTE: "H" ESTABLISHED ELLIP<br>NOTE: "h" ESTABLISHED GPS N  | TUDE/LONGITUDE IN NETWORK.<br>SOID HEIGHT IN NETWORK. 90501 RE<br>AVD 88 ORTHOMETRIC HEIGHT IN NET  | PT03.TXT, DATED 2006.11.27.<br>TWORK.  |  |  |  |
| FINAL NETWORK ADJUSTMENT RE  | PORT: 90501_RPT12.TXT, DATED 200  | 06.11.28.  |  |  |  |

FDOT D5 PID

PNC STATION REPORT FDOT D5 LOCATION SURVEY

Thursday, January 11, 2007 2:47:55 P

90501007

DUE TO THE USE OF AUTOMATED IMPORTATION PROCESSES INTO THE PNC DATABASE THE PUBLISHED VALUES AS DEPICTED IN THIS REPORT MAY BE EXPRESSED AT A HIGHER PRECISION THAN ACTUALLY DETERMINED. PLEASE REFER TO THE POSITIONAL QUALITY STATEMENT FOR EACH PUBLISHED COMPONENT TO DETERMINE APPROPRIATE USAGE PRECISIONS.

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| FLORIDA DEPARTMENT OF TRANSPORTATION DISTRICT FIVE LOCATION SURVEY SECTION<br>STATION SITE OBSTRUCTION MASKING REPORT |                                |                |  |                         |  |  |
|---|--------------------------------|----------------|--|-------------------------|--|--|
| PAGE 1 OF   |                                |                |  |                         | DATE: 2005. <u>/2</u> . <u>2/</u>                              |  |
| FIELD PARTY:  | J. HAWK                        | EN ,           | F. TOYLOR, A.                            | NEWCOM                  | B  |  |
| FULL NETWORK  | STATION NAME:                  | 09             | 05 01                                    | 007                     | FDOTD5 PID: 90501007   |  |
| HAND HELD CLI   | NOMETER: YES                   | L              | NO                                       | HAND                    | ) HELD COMPASS: YES <u> </u>                                   |  |
| OTHER   |                                |                |  |                         |  |  |
| H.I. MEASURED   | WITH: FOL                      | DIN            | JG- RULS                                 |                         | 2  |  |
| OBSTRUCTIONS  | OBSERVED AT H                  | .1. 0          | DF 1.790 m ABOV                          | E NORMAL G              | ROUND AT STATION SITE.   |  |
| STATION MARK  | IS 0.00 M                      | ETERS          | BELOW                                    | NORMAL                  | GROUND AT STATION SITE.  |  |
| H = AZIMUTH A<br>V = ANGLE OF   | NGLE FROM MAG<br>ELEVATION FRO | NETIC<br>M THE | C NORTH AS DETERMI<br>E APPARENT HORIZON | NED BY HAN<br>AT THE IN | ND COMPASS. FULL CIRCLE SWEEP.<br>ISTRUMENT OBSERVATION HEIGHT |  |
| BEGIN OBSTR   | UCTION                         |                | END OBSTRUCTIO                           | N                       | OBSTRUCTION DESCRIPTION  |  |
| <u> </u>  | <u>15</u> v                    | TO             | _360_н_/                                 | 5_v_                    | NO OBSTRUCTION   |  |
| H   | v                              | TO             | H  | V                       |  |  |
| H   | v                              | TO             | Н  | V                       |  |  |
| H   | v                              | то             | H  | V                       |  |  |
| H   | V                              | то             | H  | V                       |  |  |
| H   | v                              | то             | H  | V                       |  |  |
| H   | V                              | TO             | H  | V                       |  |  |
| H   | v                              | ТО             | H  | V                       |  |  |
| H   | V                              | TO             | <u>н</u>                                 | V                       | £  |  |
| H   | V                              | то             | H  | V                       |  |  |
| H   | V                              | TO             | H  | V                       |  |  |
| H   | V                              | TO             | H  | V                       |  |  |
| H   | V                              | TO             | Н  | V                       |  |  |
| H   | V                              | TO             | H  | V                       |  |  |
| H   | V                              | TO             | Н  | V                       | · · · · · · · · · · · · · · · · · · ·                          |  |
| H   | v                              | TO             | H  | v                       |  |  |
| H   | V                              | то             | н  | V                       |  |  |
| COMMENTS  |                                |                |  |                         |  |  |

| FDOT D5 PID<br>VOLV119  | PNC<br>FDO<br>PNC  | STATION REPORT<br>T D5 LOCATION SURVE<br>WORK GROUP                 | Thursday, Ja<br>Y  | anuary 11, 2007 2:33:12 P   |       |
|---|--|---|--|---|-------|
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| DUE TO THE USE OF AUTOM.<br>THIS REPORT MAY BE EXPRI<br>QUALITY STATEMENT FOR E | ATED IMPORTATION PRO<br>ESSED AT A HIGHER PRE<br>ACH PUBLISHED COMPO | CESSES INTO THE PNC<br>CISION THAN ACTUALLY<br>NENT TO DETERMINE AF | DATABASE THE PUBLIS<br>7 DETERMINED. PLEASE<br>PPROPRIATE USAGE PR | HED VALUES AS DEPICTED IN<br>REFER TO THE POSITIONAL<br>ECISIONS. |       |
| HECTOR_NAME VOLV119   | EFB  |   | 3W<br>GC   |   |       |
|   | TUDE   | DESTROYED STATI   |  |   | Γ     |
|   | 39 47420   | ELE STATION   |  |   | ·     |
|   | 11_51040   |   |  |   |       |
|   | 11.51242   |   |  | NGITODE N/A N/A N/A   |       |
| PPHNC BY GPS NETWORK  | OBSERVATIONS   |   |  | 10DE7 LONGITUDE REFERENCE L                                       | DATUM |
| LATITUDE / LONGITUDE  | PHNC 90501. 90501_F  | RPT12.TXT, DATED 2  | 006.11.28.   |   |       |
|   |  |   |  |   |       |
| -29.2394  | -8.  | 9122  | NAD 83   |   |       |
| ELLIPSOID HEIGHT POSITION   | AL QUALITY PPHNC B   | Y GPS NETWORK OBSE  | RVATIONS   |   |       |
| ELLIPSOID HEIGHT<br>PUBLICATION<br>SOURCE                                       | 90501. 90501_RPT12   | .TXT, DATED 2006.3  | 11.28.   |   |       |
| SPC - NORTHING US SVY FEI   | ET SPC - NORTHING  | METERS STAT   | E PLANE PROJECTION S   | CALE FACTOR   |       |
| 1706489.9135  | 520139.1659  | .999  | 95443  |   |       |
| SPC - EASTING US SVY FEE  | T SPC - EASTING M  | ETERS STAT  | E PLANE ZONE   |   |       |
| 548618.7098   | 167219.3172  | FLOF  | RIDA EAST  |   |       |
| STATE PLANE COORDINATE  | POSITIONAL QUALITY   | PPHNC BY GPS NETWO  | RK OBSERVATIONS  |   |       |
| STATE PLANE PPHNC 905<br>PUBLICATION<br>SOURCE                                  | ;01. 90501_RPT12.TX  | T, DATED 2006.11.:  | 28.  |   |       |
| DIFFERENTIALLY LEVELED<br>ORTHOMETRIC HEIGHT                                    | DL ORTHOMETRIC HEIGH   | IT IN US SVY FEET DL  | ORTHOMETRIC HEIGHT   | IN METERS   | DATUM |
| GPS DERIVED<br>ORTHOMETRIC HEIGHT   | GPS ORTHO HEIGHT IN U<br>62.3942                                     | IS SVY FEET GF  | PS ORTHO HEIGHT IN ME  | TERS NAVD 88  |       |
| ORTHOMETRIC HEIGHT<br>POSITIONAL QUALITY  | PROXIMATE BY GPS N   | ETWORK OBSERVATION  | IS   |   |       |
| ORTHOMETRIC HEIGHT<br>PUBLICATION SOURCE  | HNC 90501. 90501_R   | PT12.TXT, DATED 20  | 006.11.28.   |   |       |
| ELEVATION ELE   | A A A A A A A A A A A A A A A A A A A                                | ELEVATION   | IN METERS  | ELEVATION DATUM   |       |
| ELEVATION<br>POSITIONAL QUALITY   | A  |   |  |   |       |
| ELEVATION N/  | A  |   |  |   |       |
| PUBLICATION SOURCE  |  |   |  |   |       |

| FDOT D5 PID PNC FDOT VOLV119 PNC   | STATION REPORT<br>D5 LOCATION SURVEY   | Thursday, January 11, 2007 2:33:12 P   |
|--|--|--|
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| GPS OBSERVATION POTENTIAL POOR   |  |  |
| STATION VISITATION SAFETY CONSIDERATION  |  |  |
| NO OBSERVED EXTRAORDINARY SAFETY CONSIDE   | RATIONS REPORTED FOR T   | HIS STATION SITE.  |
| DESCRIPTION OF STATION MONUMENT - DISK / CAP A   | ND STAMPINGS   |  |
| MONUMENT CONSTRUCTION: 10" DIAMETER POUF<br>MONUMENT CONDITION: GOOD<br>DISK/CAP LOCATION ON MONUMENT: CENTERED<br>DISK/CAP SETTING: SET WHEN CAST<br>DISK/CAP CONSTRUCTION: 3" DIAMETER STEIN<br>DISK/CAP COCUPATION POINT: DIMPLE IN CEN<br>DISK/CAP CASTING STAMP: [COUNTY OF VLOUS<br>DISK/CAP HAND STAMPING: [V119]<br>DISK/CAP STABLITY: GOOD<br>DISK/CAP STABLITY: GOOD | ED IN PLACE CONCRETE M<br>ON TOP<br>MAN ALUMINUM SURVEY DI:<br>TER OF RAISED TRIANGLE<br>IIA] [1991 GPS CONTROL]               | onument<br>SK<br>[LB1701] [STEINMAN & ASSCO]   |
| STATION SITE "TO REACH" DESCRIPTION  |  |  |
| PPHCN 90501 VOLV119 FIELD STATION REPORT<br>AT THE INTERSECTION OF S.R. 44 (NEW YORK<br>ALONG S.R. 44 (NEW YORK AVE.) FOR 0.7 MI<br>BETWEEN S.R. 44 (NEW YORK AVE.) AND OLD<br>PAVEMENT OF S.R. 44 (NEW YORK AVE.).  | , DATED 2005.12.19. (A:<br>AVE.) AND S.R. 15A (SI<br>LES TO THE STATION ON S<br>NEW YORK AVENUE AND IS                         | LL DISTANCES ARE APPROXIMATE)<br>PRING GARDEN AVE.), PROCEED WESTERLY<br>THE LEFT. STATION IS IN A GRASS MEDIAN<br>16.9 FT. SOUTH OF THE SOUTH EDGE OF |
| REFERENCE OBJECTS:         PPHCN 90501 VOLV119 F           S 42 W         61.2 FT.         N-EOP, OLD NEW YORK F           S 85 E         31.7 FT.         POWER POLE, WOOD, #77           N 05 E         16.9 FT.         S-EOP, S.R. 44 (NEW Y           N 76 W         32.2 FT.         MITRED END SECTION, N           S 05 W         3.0 FT.         COMPOSITE RESIN POST | IELD STATION REPORT, D<br>NE. PK NAIL AND PCP [L<br>5606, MAG NAIL AND [FD<br>ORK AVE.) MAG NAIL AND<br>W-CORNER<br>WITH DECAL | ATED 2005.12.19.<br>31701]<br>DT REF.] DISK<br>[FDOT REF.] DISK  |
| SURVEY BASELINE STATION PLUS SURVEY BASEL  | INE STATION OFFSET US S  | VY FT METERS   |
|  |  |  |
| J  |  |  |
| STATION HISTORY ASSOCIATED   | MEDIA FILES AVAILABLE ON   | CD.  |
| 2007.01.11 BY D. SARACINAS OF FDOT D5 ST<br>INITIAL PUBLICATION SOURCE IS THE 1990 V<br>GPS AZIMUTH MARK, GPS STATION DATA FORM,<br>THIS IS A NON BLUE BOOKED AZIMUTH STATIC<br>STATION MONUMENTED: SEPTEMBER 1991. (STE<br>STATION NAME : V119<br>STATION ALIAS: UNKNOWN  | AFF, REPORTING ON INIT<br>OLUSIA COUNTY DENSIFIC<br>STEINMAN & ASSOCIATES<br>N.<br>INMAN & ASSOC.)                             | IAL PUBLICATION SOURCE.<br>ATION PROJECT BY KEITH AND SCHNARS.   |
| LATITUDE : ESTABLISHED. BY GPS OBSERVATI<br>LONGITUDE: ESTABLISHED. BY GPS OBSERVATI<br>ELLIPSOID HEIGHT: NOT REPORTED.<br>GPS NGVD 29 ELEVATION: NOT REPORTED.<br>GPS NAVD 88 ORTHOMETRIC HEIGHT: NOT REPO<br>LEVELED NGVD 29 ELEVATION: NOT REPORTED<br>LEVELED NAVD 88 ORTHOMETRIC HEIGHT: NOT  | ONS. NAD 83 (1990).<br>ONS. NAD 83 (1990).<br>WRTED.<br>REPORTED.  | *****  |
| 2006.03.01 BY J. HAWKINS OF FDOT D5 STAF<br>STATION RECOVERED: YES. DECEMBER 2005. (<br>STATION OCCUPIED : YES.<br>NETWORK NAME : 90501 VOLV119  | F, REPORTING ON PPHCN (<br>FDOT)   | 90501, F.P. NO.4046461.  |

| FDOT D5 PID PNC  | STATION REPORT<br>T D5 LOCATION SURVEY<br>WORK GROUP                              | Thursday, January 11, 2007 2:33:12 P  |
|--|---|---|
| DATA PROVIDED IS FOR INFORMATIONAL PURPOS<br>TO THE USER. USER ASSUMES ALL RISK AND LIAB   | ES ONLY. DATA IS SUBJEC<br>BILITY FROM DATA USAGE                                 | CT TO CHANGE AND REVISION WITHOUT NOTICE<br>UNLESS AUTHORIZED OTHERWISE BY FDOT.                                      |
| DUE TO THE USE OF AUTOMATED IMPORTATION PRO<br>THIS REPORT MAY BE EXPRESSED AT A HIGHER PRE<br>QUALITY STATEMENT FOR EACH PUBLISHED COMPO  | CESSES INTO THE PNC DA<br>CISION THAN ACTUALLY D<br>NENT TO DETERMINE APPI        | TABASE THE PUBLISHED VALUES AS DEPICTED IN<br>ETERMINED. PLEASE REFER TO THE POSITIONAL<br>ROPRIATE USAGE PRECISIONS. |
| STATION ALIAS: V119 STEINMAN & ASSOC.<br>EFB NAME: N/A   |   |   |
| LATITUDE : UNCHANGED.<br>LONGITUDE: UNCHANGED.<br>ELLIPSOID HEIGHT: ESTABLISHED. GPS OBSE<br>GPS NGVD 29 ELEVATION: N/A<br>GPS NAVD 88 ORTHOMETRIC HEIGHT: ESTABLI<br>LEVELED NGVD 29 ELEVATION: N/A<br>LEVELED NGVD 88 ORTOMETRIC HEIGHT: N/A | RVATIONS.<br>SHED.  |   |
| NOTE: STATION PHOTOGRAPHY TAKEN  |   |   |
| NOTE: "XY" HELD LATITUDE/LONGITUDE FIXE<br>NOTE: GPS STATION DATA SHEET, DATED DEC<br>NOTE "H" ESTABLISHEDD ELLIPSOID HEIGHT<br>NOTE "h" ESTABLISHED GPS NAVD 88 ORTHOM  | D PER 1990 VOL. CO.<br>EMBER 1991.<br>IN NETWORK. 90501_R<br>ETRIC HEIGHT IN NETW | DENSIFICATION PROJECT, STEINMAN & ASSOC.<br>PT03.TXT, DATED 2006.11.27.<br>WORK. 90501_RPT12.TXT, DATED 2006.11.28.   |
| FINAL NETWORK ADJUSTMENT REPORT: 90501   | RPT12.TXT, DATED 200  | 06.11.28.<br>*******  |

| FLORII  | DA DEPARTMEN  | r of<br>Stat:  | TRANSPORT | ATIO  | N DISTRI                               | CT FIN | VE LOCATION SURVEY SECTION<br>IG REPORT                          |
|---|---------------|----------------|-----------|-------|--|--------|--|
| PAGE 1 OF DATE: 2005. ] Z . ] S                                   |               |                |           |       |  |        |  |
| FIELD PARTY: J, HAWKIH, F. TAYIDR, A. NEWCOMB                     |               |                |           |       |  |        |  |
| FULL NETWORK STATION NAME: 09 05 01 VOL.V/19 FDOTD5 PID: VOL V/19 |               |                |           |       |  |        |  |
| HAND HELD CLIN  | NOMETER: YES  | i-             | NO        |       |  | IAH    | ND HELD COMPASS: YES NO  |
| OTHER   |               |                |           |       |  |        |  |
| H.I. MEASURED   | WITH:         |                |           |       |  |        |  |
| OBSTRUCTIONS C  | DBSERVED AT 1 | H.I.           | OF 1.790  | m     | ABOVE N                                | ORMAL  | GROUND AT STATION SITE.  |
| STATION MARK  | IS 0.00 I     | METER          | S ABOVE _ | BI    | ELOW                                   | NORMA  | AL GROUND AT STATION SITE.                                       |
| H = AZIMUTH AN<br>V = ANGLE OF H                                  | GLE FROM MAG  | GNETI<br>OM TH | C NORTH A | S DE' | FERMINED<br>RIZON AI                   | BY HA  | AND COMPASS. FULL CIRCLE SWEEP.<br>INSTRUMENT OBSERVATION HEIGHT |
| BEGIN OBSTRU  | JCTION        |                | END O     | BSTR  | JCTION                                 |        | OBSTRUCTION DESCRIPTION  |
| н   | <u> </u>      | TO             | 5         | H     | 30                                     | V      | TREELINE   |
| <u>5</u> <sup>H</sup>   | _15v          | TO             | 12        | H     | 15                                     | v      | NO OBSTRUCTION   |
| НН  | v <u>52</u>   | TO             | 8Z        | H     | 22                                     | v _    | TREELINE   |
| 82 н  | <u>15</u> v   | TO             | 110       | H     | 15                                     | v      | NO OBSTRUCTION   |
| 110 н   | <u> </u>      | TO             | 270       | H     | 26                                     | v _    | TREE LINE  |
| H OFS   | <u>15</u> v   | TO             | 297       | H     | 15                                     | v      | NO OBSTRUCTION   |
| _297_н  | <u>24</u> v   | TO             | 323       | H     | 24                                     | v _    | TREE LINE  |
| <u>ЗСЗ</u> н  | <u>15</u> v   | TO             | 330       | H     | 15                                     | V      | NO OBSTRUCTION   |
| <u>330</u> H  | <u>30 </u> v  | TO             | 360       | H     | 30                                     | V      | TREE LINE  |
| H   | V             | TO             |           | H     |  | v      |  |
| H   | V             | TO             |           | H     |  | V      |  |
| H   | V             | TO             |           | H     |  | V      |  |
| H   | V             | TO             | -         | H     | <del>.</del>                           | V      |  |
| Н   | V             | TO             |           | H     |  | V      |  |
| H   | V             | TO             | -         | H     |  | V      |  |
| H   | V             | TO             |           | H     |  | V      |  |
| H   | V             | TO             |           | H     | ( <del>7 - 2 - 2 - 1 - 1 - 2 - 2</del> | V      |  |
|   |               |                |           | COL   | MENTS                                  |        |  |
|   |               |                |           |       |  |        |  |
|   |               |                |           |       |  |        |  |
|   |               |                |           |       |  |        |  |
| 1-1   |               |                |           |       |  |        |  |
|   |               |                |           |       |  |        | 3  |

F

## **APPENDIX B: PLANNING MATERIAL**



Planning for accuracy determination data collection for 50-second collection time on January 17, 2011







Planning for distance accuracy data collection on January 24, 2012



Planning for Scenario 1A data collection on February 11, 2012



Planning for Scenario 1B data collection on February 12, 2012



Planning for Scenario 2 data collection on February 16, 2012



Planning for Scenario 3 data collection on January 29, 2012



Planning for Scenario 4 data collection on February 20, 2012



Planning for Scenario 5 data collection on February 2, 2012



Planning for Scenario 6 data collection on March 31, 2012



Planning for Scenario 7 data collection on March 17, 2012



Planning for Scenario 8 data collection on March 18, 2012



Planning for Scenario 9 data collection on March 31, 2012

## **APPENDIX C: ACCURACY MAPS**
















## **APPENDIX D: SCENARIO MAPS**





















































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